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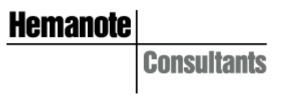
# 400-404 CABRAMATTA ROAD WEST, 2-18 ORANGE GROVE ROAD & 6 LINKS AVENUE, CABRAMATTA

PROPOSED RESIDENTIAL FLAT BUILDING AND TOWNHOUSES

# UPDATED TRAFFIC & PARKING IMPACT ASSESSMENT

MAY 2023

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# TRAFFIC & PARKING IMPACT ASSESSMENT 400-404 CABRAMATTA ROAD WEST, 2-18 ORANGE GROVE ROAD & 6 LINKS AVENUE, CABRAMATTA PROPOSED RESIDENTIAL FLAT BUILDING AND TOWNHOUSES DATE: 23 MAY 2023

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# **1 INTRODUCTION**

This report has been prepared by Hemanote Consultants to assess the traffic and parking implications of the proposed residential flat building and townhouses development to be located at **400-404 Cabramatta Road West**, **2-18 Orange Grove Road and 6 Links Avenue, Cabramatta**, accommodating eighty-seven (87) residential apartments over two basement parking levels and fifty-three (53) townhouses with at-grade and basement parking.

This report is to be read in conjunction with the architectural plans prepared by Alexandar Projects and Designiche Building Designers (reduced copy of the architectural plans are attached in Appendix 'A' of this report) and submitted to Fairfield City Council as part of a Development Application.

This report is set as follows:

- Section 2: Description of the existing site location and its use;
- Section 3: Description of existing traffic conditions near the subject site;
- Section 4: Description of the proposal, vehicular access, on-site parking provision, layout and circulation;
- Section 5: Assessment of impacts on parking;
- Section 6: Assessment of impacts on traffic in the vicinity of the subject site; and
- Section 7: Outlines conclusions.

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# 2 EXISTING SITE DESCRIPTION

#### > Site Location

The subject site is located on the southern side of Cabramatta Road West at properties No. 400, 402, 402A and 404 (legally known as Lot 1 of DP29449, Lots 1 and 2 of DP503339, and Lot 6 of DP709126), on the eastern side Orange Grove Road at properties No. 2-18 (legally known as Lot 7 of DP709126), and on the northern side of Links Avenue at property No. 6 (legally known as Lot 3 of DP30217), within the suburb of Cabramatta. The site has a primary frontage of 49.9 metres to Cabramatta Road West from the north, a secondary frontage of approximately 185.8 metres to Orange Grove Road from the west and a third frontage of 14.9 metres to Links Avenue from the south. Refer to Figure 1 for a site locality map.

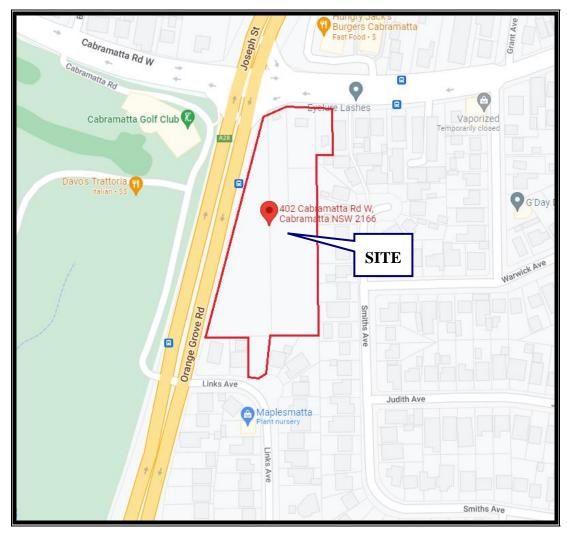


Figure 1: Site Locality Map

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#### > Existing Site & Surrounding Land Use

The subject site has a total area of 15,349m<sup>2</sup> and is currently occupied by vacant land. It is located in a mixed residential and commercial area, characterised by residential dwellings and is in close proximity to some retail and commercial sites. The site is located approximately 2 km from Cabramatta Railway Station and 3.2 km from Warwick Farm Railway Station.



Photo 1: Site frontage to Cabramatta Road West





Photo 2: Site frontage to Orange Grove Road



Photo 3: Site frontage to Links Avenue

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# **3 EXISTING TRAFFIC CONDITIONS**

#### 3.1 Road Network and Classification

Cabramatta Road West is a state road that generally runs in an east to west direction, between Railway Parade (regional road) to the east and Elizabeth Drive (state road) to the west. Cabramatta Road West intersects with Orange Grove Road (state road) at the subject site.

#### 3.2 Road Description and Traffic Control

Cabramatta Road West has a two-way divided carriageway with a width between kerbs of approximately 21 metres. This carriageway generally provides two travel lanes per direction. At present, parking is not permitted along Cabramatta Road West in the vicinity of the subject site, as per the signposted 'Clearway' between 6.00am and 7.00pm Mondays to Fridays and between 9.00am and 6.00pm on weekends and public holidays, as well as the signposted 'No Stopping' in the vicinity of the subject site.

The legal speed limit on Cabramatta Road West is signposted at 60km/h, with the exception of the signposted 'School Zone' of 40km/h during morning and afternoon school peak periods. Cabramatta Road West intersects with Orange Grove Road at the subject site and is controlled by traffic signals.



Figure 2: Aerial photograph of the subject site and surrounding road network

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Photo 3: Cabramatta Road West at the subject site - facing east



Photo 4: Cabramatta Road West at the subject site - facing west

Orange Grove Road has a two-way divided carriageway, with a width between kerbs of approximately 24 metres. This carriageway generally provides three travel lanes per direction. At present, parking is not permitted on Orange Grove Road in the vicinity of the subject site, as per the signposted 'Clearway' between 6.00am and 7.00pm Mondays to Fridays and between 9.00am and 6.00pm on weekends and public holidays, as well as the signposted 'No Stopping' and 'No Parking' in front of the subject site. The legal speed limit on Orange Grove Road is signposted at 70km/h.

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Photo 5: Orange Grove Road at the subject site - facing north



Photo 6: Orange Grove Road at the subject site - facing south

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Links Avenue has a two-way undivided carriageway, with a width between kerbs of approximately 9 metres. This carriageway generally provides one travel lane per direction with kerbside parking available on both sides of the road. At present, unrestricted parking is permitted on Links Avenue, with the exception of the signposted 'No Stopping' at its near intersection with Orange Grove Road. The legal speed limit on Links Avenue is signposted at 50km/h.



Photo 7: Links Avenue at the subject site - facing south-east



Photo 8: Links Avenue at the subject site - facing west

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#### 3.3 Current Traffic Flows

A traffic volume count was undertaken by R.O.A.R. DATA Pty Ltd on behalf of Hemanote Consultants at the intersection of Orange Grove Road / Links Avenue / Access Road to Golf Club in the vicinity of the subject site on Tuesday 9 August 2022, during morning period (7.00am to 9.00am) and afternoon period (3.00pm to 6.00pm), considering traffic peak periods.

The current traffic flows in the morning & afternoon peak hours are shown in Table 1 below and Appendix 'C' of this report.

Traffic movement	Morning Peak Hour (Vehicles Per Hour)	Evening Peak Hour (Vehicles Per Hour)	
	7.45am – 8.45am	3.45pm – 4.45pm	
(	Orange Grove Road (North of Links Ave	enue) – 3 lanes	
Northbound	1,529	1,924	
Southbound	1,715	1,851	
(	Drange Grove Road (South of Links Av	enue) – 3 lanes	
Northbound	1,512	1,922	
Southbound	1,716	1,824	
	Links Avenue (East of Orange Gro	ove Road)	
Eastbound	11	38	
Westbound	32	16	
A	ccess Road to Golf Club (West of Orar	nge Grove Road)	
Eastbound	4	2	
Westbound	7	5	

Table 1: Current Peak traffic flows in the vicinity of the subject site (on a typical weekday)

The results of the traffic volume counts undertaken determined that the traffic morning peak period was between 7.45am to 8.45am and the afternoon peak period was between 3.45pm to 4.45pm on a typical weekday.

The existing traffic flows on Orange Grove Road are typical for a state road in a mixed residential and commercial area.

The existing traffic flows on Links Avenue are low and typical for a local residential street, where traffic is controlled by the existing signalised intersection with Orange Grove Road.

It is determined that the existing mid-block level of service on Links Avenue is it level 'A', in accordance with Table 4.4 of the Roads & Maritime Services' *"Guide to Traffic Generating Developments - 2002"*, where peak hour flow is less than 200 vehicles/hr.

Level of Service	One Lane (veh/hr)	Two Lanes (veh/hr)
А	200	900
В	380	1400
С	600	1800
D	900	2200
E	1400	2800

Table 4.4: Urban road peak hour flows per direction RMS Guide)

#### > Current Intersection Performance

Average Vehicle Delay (AVD) and Level of Service (LOS) – The AVD and LOS provide a measure of the operational performance of an intersection, as indicated in Table 4.2 of the Roads & Maritime Services "*Guide to Traffic Generating Developments - 2002*" (shown below).

Level of Service	Average Delay per Vehicle (secs/veh)	Traffic Signals, Roundabout	Give Way & Stop Signs
А	< 14	Good operation	Good operation
В	15 to 28	Good with acceptable delays & spare capacity	Acceptable delays & spare capacity
С	29 to 42	Satisfactory	Satisfactory, but accident study required
D	43 to 56	Operating near capacity	Near capacity & accident study required
E	57 to 70	At capacity; at signals, incidents will cause excessive delays	At capacity, requires other control mode
		Roundabouts require other control mode	

Table 4.2: Level of Service Criteria for intersections (RMS Guide)

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A SIDRA intersection performance analysis was undertaken for the existing crossintersection of Orange Grove Road / Links Avenue in the vicinity of the subject site (Pre-development).

Refer to Figure 3 below, showing the existing cross-intersection layout controlled by traffic signals.

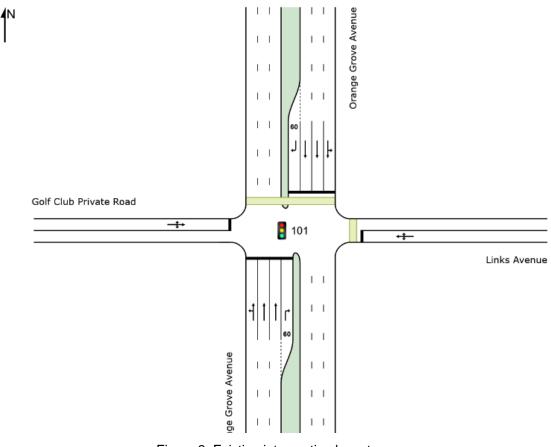


Figure 3: Existing intersection layout

The SIDRA performance analysis determined that the current operational performance during the AM peak period for the Orange Grove Road north approach leg is generally operating near capacity at a level of service 'D', with the exception of the right turn movement from Orange Grove Road into Links Avenue which is at level 'E'.

The existing operational performance during the AM peak period of the Orange Grove Road south approach is generally in good operation at a level of service 'A'. However, the left turn movement from Orange Grove Road into Golf Club Private

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Road is at a level of service 'B' and the right turn movement from Orange Grove Road into Links Avenue is at level 'C'.

The existing operational performance during the AM peak period of the Links Avenue east approach leg is at a level of service 'D'.

The SIDRA performance analysis determined that the current operational performance during the PM peak period for the Orange Grove Road north approach leg is generally operating near capacity at a level of service 'E', with the exception of the right turn movement from Orange Grove Road into Links Avenue which is at level 'F'.

The current operational performance during the PM peak period of the Orange Grove Road south approach is generally in good operation at a level of service 'A'. However, the left turn movement from Orange Grove Road into Golf Club Private Road is at a level of service 'B' and the right turn movement from Orange Grove Road into Links Avenue is at level 'C'.

The current operational performance during the PM peak period of the Links Avenue east approach leg is at a level of service 'E'.

Refer to the summary of the results of the SIDRA intersection performance analysis attached in Appendix 'D' of this report.

#### 3.4 Existing Transportation Services

The subject site has good access to existing public transport services in the form of trains and buses. The site is located approximately 2 km from Cabramatta Railway Station and approximately 3.2 km from Warwick Farm Railway Station.

Frequent bus services operate along Cabramatta Road West and Orange Grove Road, in close proximity to the subject site (i.e. bus routes 801, 815, 816 and 819).

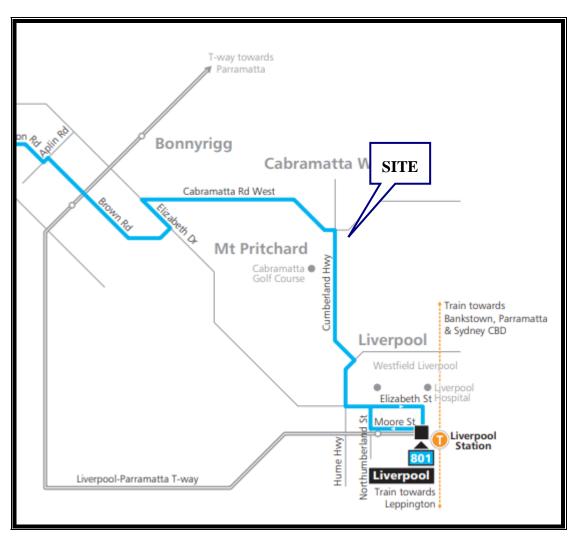


Figure 3: Bus services near the subject site (Bus no. 801)

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Existing Bus stations near the subject site

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# 4 PROPOSED DEVELOPMENT

## 4.1 Description of the proposal

The proposal is for the construction of a residential flat building and townhouses development, to be located at 400-404 Cabramatta Road West, 2-18 Orange Grove Road and 6 Links Avenue, Cabramatta, with at-grade and basement parking levels.

The proposed development will include the following:

- A Residential Flat Building, with a total of eighty-seven (87) residential units consisting of:
  - 30 x 1-bedroom apartments.
  - 56 x two-bedroom apartments (including 9 adaptable units).
  - 1 x three-bedroom apartment.
- A total of one-hundred and nine (109) on-site car parking spaces, with 87 spaces for resident parking (including 9 adaptable parking spaces) and 22 spaces for visitor parking (including 3 accessible spaces with adjacent shared areas and a car wash bay), in addition to forty-four (44) bicycle storage spaces and a turning bay, over two basement parking levels and 9 visitor bicycle spaces on ground floor. A truck loading bay is located externally on ground level, accommodating up to a Heavy Rigid Vehicle (HRV 12.5 metres in length).
- Fifty-three (53) residential townhouse dwellings consisting of:
  - 34 x three-bedroom townhouses (including 6 adaptable dwellings).
  - 19 x four-bedroom townhouses.
- A total of one-hundred and fifty-four (154) on-site car parking spaces allocated for the townhouses, with 59 open spaces and 48 enclosed garages (including 1 double garage and 6 adaptable garages) located on ground level, and 47 car spaces in basement level. A total of 106 car spaces are allocated for residents (2 car spaces per townhouse) and 48 car spaces for visitors.

Refer to *Appendix 'A'* for the proposed development plans.

#### 4.2 Vehicular & Pedestrian Access

The vehicular access to and from the off-street parking facilities will be via a new access driveway crossing to be located in Links Avenue. The access driveway is to have a width of 7.7 metres, which is adequate for a low volume (Category 1) access driveway in accordance with AS2890.1:2004 and waste collection truck access.

The access driveway is to provide two-way vehicular movements, where two vehicles can pass each other at the same time without causing delays or congestion to traffic on the street. The proposed access driveway is located more than 6 metres from the tangent point of the adjacent kerbline, in accordance with Figure 3.1 of AS2890.1:2004.

Vehicular access is to be located and constructed in accordance with the requirements of AS2890.1:2004, where vehicles enter and exit the site in a forward direction at all times.

The existing vehicular crossings located in Orange Grove Road and Cabramatta Road West are to be removed and replaced with new kerb, gutter and footpath, to be constructed to Council specifications, to restore on-street car parking spaces.

The clear sight line triangle (2.5m x 2m) between the driver's eye view and pedestrians is to be provided on the exit side of the driveway, as per Figure 3.3 of AS2890.1:2004 and Figure 3.4 of AS2890.2:2018. Separate pedestrian access gates are also provided at the front of the site in Cabramatta Road West and Orange Grove Road, to segregate pedestrians and vehicles and improve safety within the site.

## 4.3 On-site Parking Provision

Fairfield CityWide Development Control Plan 2013, Chapter 12, Section 12.1, Table 1, requires off-street parking for <u>multi-dwelling housing</u> located less than 400m from a major bus station to be provided at a rate of:

- 1-2 bedrooms (less than 110m<sup>2</sup>) 1 space.
- 3 or more bedrooms (greater than 110m<sup>2</sup>) 1.5 spaces.
- 0.25 visitor spaces per dwelling.

Refer to Table 2 below for the required and proposed car parking provision for the proposed **townhouses**:

Proposed breakdown	Car parking rate	Proposed No. of units	Car parking required	Total car parking required	Total car parking provided	
		Multi-Dwellin	g Housing			
1-2 bed apartments (less than 110m <sup>2</sup> )	1 space	-	-	94		
3 or more bed apartments (greater than 110m <sup>2</sup> )	1.5 spaces	53	80		154	
Visitor	0.25 spaces per dwelling	53	14			
				94	154	
Compliance with on-site car parking				Yes		

Table 2: On-site parking requirement and provision (Townhouses)

The proposed development provides for fifty-three (53) residential dwellings (including 34 x 3-bedroom dwellings and 19 x 4-bedroom dwellings), and therefore requires a minimum of 94 car parking spaces, as outlined above, including a minimum of 14 visitor car spaces and 80 resident car spaces.

The proposed development provides one-hundred and fifty-four (154) on-site car parking spaces allocated for the townhouses, with 59 open spaces and 48 enclosed garages (including 1 double garage and 6 adaptable garages) located on ground

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level, and 47 car spaces in basement level. A total of 106 car spaces are allocated for residents (2 car spaces per townhouse) and 48 car spaces for visitors.

Fairfield CityWide Development Control Plan 2013, Chapter 12, Section 12.1, Table 1, requires off-street parking for <u>residential flat buildings</u> to be provided at a rate of:

- 1 space per dwelling, plus
- 1 visitor space per 4 dwellings where a development has more than 2 proposed dwellings.

Refer to Table 3 below for the required and proposed car parking provision for the proposed **RFB**:

Proposed breakdown	Car parking rate	Proposed No. of units	Car parking required	Total car parking required	Total car parking provided
		Residential F	at Building		
Dwellings	1 space per dwelling	07	87	109	109
Visitor	1 space per 4 dwellings	87	22	109	109
Total 109					109
Compliance with on-site car parking			Yes		

Table 3: On-site parking requirement and provision (RFB)

The proposed development provides for eighty-seven (87) residential units (including  $30 \times 1$ -bedroom units,  $56 \times 2$ -bedroom units and  $1 \times 3$ -bedroom units), and therefore requires a total of 109 car parking spaces.

The proposed development provides one-hundred and nine (109) on-site car parking spaces, with 87 spaces for resident parking (including 9 adaptable parking spaces) and 22 spaces for visitor parking (including 3 accessible spaces with adjacent shared areas and a car wash bay), in addition to forty-four (44) bicycle storage spaces and a turning bay, over two basement parking levels and 9 visitor bicycle spaces on ground floor. A truck loading bay is located externally on ground level, accommodating up to a Heavy Rigid Vehicle (HRV – 12.5 metres in length). Therefore, the proposed onsite parking provision is adequate for the proposed development and in compliance with Council's parking requirements.

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## 4.4 On-site Parking Layout and Circulation

The layout of the on-site car parking area and manoeuvring arrangements has been designed to enhance vehicular and pedestrian access, where vehicles enter and exit the site in a forward direction, through the provision of adequate internal aisle width and turning space.

AS2890.1:2004 Parking facilities Part 1: Off-street car parking requires a minimum parking space width of 2.4 meters (for User Class 1A – long-term residential parking) and 2.5 metres (for User Class 2 - medium-term visitor parking) and a minimum length of 5.4 meters. The proposed on-site car parking spaces have a minimum width of 2.4 metres (for residential parking) and a width of 2.5 metres (for visitor parking) and a minimum length of 5.4 meters each, which is adequate.

The accessible car parking spaces have a width of 2.4 metres, in addition to an adjacent 2.4 metres wide shared area, which is adequate in accordance with AS2890.6:2009.

The adaptable car parking spaces (for the RFB) have a width of 3.8 metres, which is adequate.

The single enclosed garages (for the townhouses) have an internal width of 3.1 metres, with the exception of 6 adaptable garages which have an internal width of 3.8 metres and the double garage at grade level, which has a width of 6 metres. The double enclosed garages located within the basement have an internal width of 5.8 metres, which is adequate

The loading bay located on ground level in a central position on the site, has a clear width of 3.5 metres and a length of 12.5 metres, which is adequate for HRV access in accordance with AS2890.2:2018.

Car parking spaces adjacent to walls or obstructions have been made wider than the minimum width, to accommodate full door opening in accordance with Clause 2.4.2(d) of AS2890.1:2004.

Clause 2.4.2 of AS2890.1:2004 requires a minimum aisle width of 5.8 metres for twoway aisles, adjacent to 90° angle parking. The proposed aisles have a <u>minimum</u>

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width of 5.8 metres, which is adequate for two-way traffic and manoeuvring into and out of parking spaces.

A turning area is also provided towards the rear of the basement carpark level for the townhouses and the basement 2 carpark level for the RFB, to allow vehicles to turn around and exit in a forward direction, if all other car parking spaces are occupied.

The ramp to the basement level of the RFB has a clear width of 6.5 metres at the top of the ramp which narrows down to a width of 5.5 metres, in addition to a 300mm kerb on either side and has a maximum grade of 1:20 (5%) for the first 6 metres within the site. It has a maximum grade of 1:4 (25%) with a change of grade of 1:8 (12.5%) over the 2 metres at either end of the ramp, which is adequate.

The ramp to the basement level for the townhouses is adequate and complies with AS2890.1:2004

A minimum 2.2 metres headroom clearance is to be provided from the car park basement levels to the underside of all services conduits and suspended stormwater pipelines, in accordance with Clause 5.3.1 of AS2890.1:2004. A "Maximum Headroom Clearance 2.2m" sign is to be erected at the entrance to the basement car park areas and is to be clearly visible to all drivers. A minimum 2.5 meters headroom clearance is to be provided above the accessible parking spaces and adjacent shared zones as well as the adaptable car parking spaces in accordance with Clause 2.4 of AS2890.6:2009.

Traffic convex mirrors are to be provided at the bottom of the vehicular ramps, to provide drivers with further assistance with viewing oncoming traffic, as an additional safety and traffic management measure.

All vehicular manoeuvring within the site has been designed and checked using the HRV, B99 and B85 standard design vehicle turning paths from AS2890.1:2004, AS2890.2:2018 and Austroads. Refer to the vehicle swept paths diagrams attached in Appendix 'B' of this report.

Therefore, the car parking layout and vehicular circulation are adequate in accordance with AS2890.1:2004, AS2890.6:2009 and AS2890.2:2018, where vehicles are to enter and exit the site in a forward direction at all times.

## 4.5 Waste Collection & Deliveries

Waste collection will be undertaken on-site within the dedicated truck loading bay located in a central position in the car parking area on ground level, by the waste collection vehicle.

The truck loading bay will also be used to service the site for ongoing deliveries and maintenance.

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5 EXTERNAL PARKING IMPACT

# 5.1 Existing Parking Controls

The subject site is located in a mixed residential and commercial area, where parking is not permitted along Cabramatta Road West in the vicinity of the subject site, as per the signposted 'Clearway' between 6.00am and 7.00pm Mondays to Fridays and between 9.00am and 6.00pm on weekends and public holidays, as well as the signposted 'No Stopping' in the vicinity of the subject site

In addition, parking is not permitted on Orange Grove Road in the vicinity of the subject site, as per the signposted 'Clearway' between 6.00am and 7.00pm Mondays to Fridays and between 9.00am and 6.00pm on weekends and public holidays, as well as the signposted 'No Stopping' and 'No Parking' in front of the subject site.

Further, unrestricted parking is permitted on Links Avenue, with the exception of the signposted 'No Stopping' at its near intersection with Orange Grove Road.

# 5.2 Impacts of Proposed Development on Parking

The parking demand resulting from the proposed residential development can be accommodated within the proposed adequate on-site car and bicycle parking spaces for residents and visitors. The subject site has great access to existing public transport in the form of trains and bus services.

Therefore, the proposed development will not have adverse impacts on parking in the surrounding area.

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# 6 EXTERNAL TRAFFIC IMPACT

## 6.1 Estimated Future Traffic Generation

An indication of the potential traffic generation of the proposed development is provided by the RMS *Guide to Traffic Generating Development - 2002*.

The Guide specifies the following traffic generation rates for **medium density** residential developments:

Larger units and town houses (three or more bedrooms):

• Weekday peak hour vehicle trips = 0.5-0.65 per dwelling.

Therefore, the proposed townhouses with fifty-three (53) residential apartments (34 x 3-bed units and 19 x 4-bed units) has an estimated traffic generation as follows:

• 27 to 35 peak hour vehicle trips (In and Out trips).

The Guide also specifies the following traffic generation rates for **high density** residential developments:

- AM peak (1 hour) vehicle trips per unit = 0.19.
- PM peak (1 hour) vehicle trips per unit = 0.15.

Therefore, the proposed RFB with eighty-seven (87) residential units (30 x 1-bed units, 56 x 2-bed units and 1 x 3-bed unit) has an estimated traffic generation as follows:

- 17 AM peak hour vehicle trips (In and Out trips).
- 13 PM peak hour vehicle trips (In and Out trips).

The total estimated peak hour traffic generation is:

- 52 AM peak hour vehicle trips (In and Out trips).
- 48 PM peak hour vehicle trips (In and Out trips).

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#### 6.2 **Projected Intersection Performance**

Average Vehicle Delay (AVD) and Level of Service (LOS) – The AVD and LOS provides a measure of the operational performance of an intersection, as indicated in Table 4.2 of the Roads & Maritime Services "*Guide to Traffic Generating Developments* - 2002" (shown below).

Level of Service	Average Delay per Vehicle (secs/veh)	Traffic Signals, Roundabout	Give Way & Stop Signs
А	< 14	Good operation	Good operation
В	15 to 28	Good with acceptable delays & spare capacity	Acceptable delays & spare capacity
С	29 to 42	Satisfactory	Satisfactory, but accident study required
D	43 to 56	Operating near capacity	Near capacity & accident study required
E	57 to 70	At capacity; at signals, incidents will cause excessive delays	At capacity, requires other control mode
		Roundabouts require other control mode	

Table 4.2: Level of Service Criteria for intersections (RMS Guide)

A post-development SIDRA intersection performance modelling analysis was undertaken for the cross-intersection of Orange Grove Road / Links Avenue in the vicinity of the subject site, and it was modelled as the proposed network layout as shown in Figure 5 on the following page.

Refer to the summary of the results of the SIDRA intersection performance analysis (undertaken for pre & post development, including the 10-year future growth) attached in Appendix 'D' of this report.

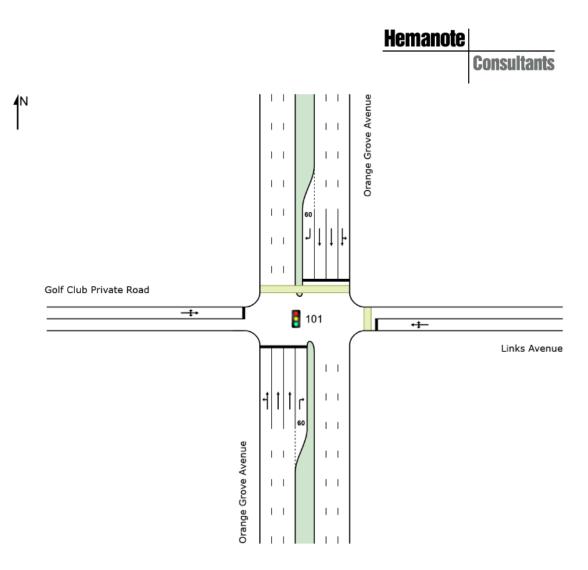


Figure 5: Intersection Layout

The following <u>assumptions</u> have been considered and adopted in the SIDRA Network Intersection modelling for the post-development conditions considering that the main access to and from the subject site is through Links Avenue:

- Morning and afternoon peak traffic generation from *RMS Guide to Traffic Generating Developments* has been used, as outlined in Section 6.1 of this report.
- AM & PM Traffic generated by the development was equally adopted for inbound and outbound traffic.
- Pre-development network analysis is modelled for the base year (2022) and 10 years of future growth (2032) in surrounding traffic. The annual traffic growth rate for the 10-year future period was based on the SIDRA intersection analysis software, which allows for future analysis in surrounding traffic by applying a uniform growth rate of 2% for each year over the 10-year period.
- Post-development network analysis is modelled for when the residential development is in operation and after 10 years of future growth in surrounding traffic.
- The entered traffic flow was inputted as 95% of the modelled flow.

These assumptions will result in the development trip distribution shown in Figures 6 and 7 for relevant traffic movement and modelled intersection.

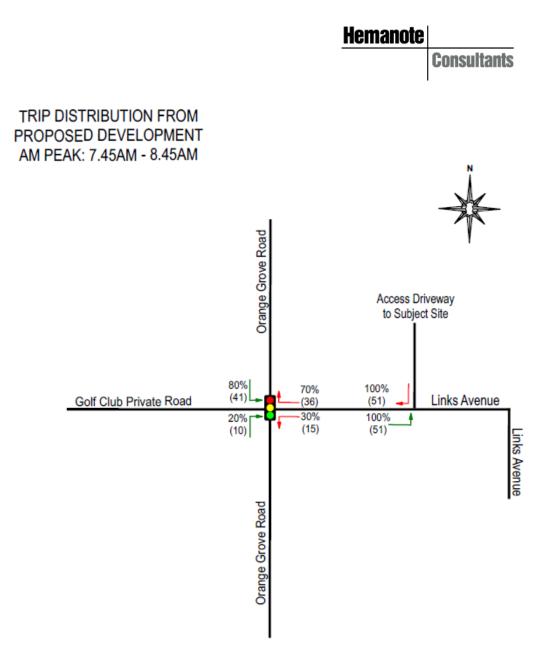


Figure 6: Development Traffic Distribution on the Surrounding Road Network - AM Peak

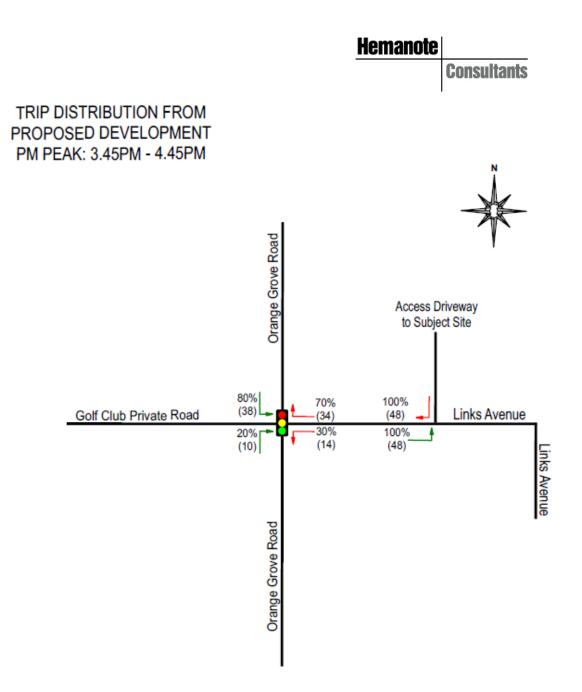


Figure 7: Development Traffic Distribution on the Surrounding Road Network - PM Peak

#### The outcome of the SIDRA modelling

A summary of the results of the SIDRA intersection performance analysis has been provided in Tables 4 and 5 below, as well as the SIDRA Movement Summary Tables attached in Appendix 'D' of this report.

Modelled Peak	Average LOS	Average Delay (sec)	DOS (Veh/C)
Base Year 2022 – Pre-Development	С	30.0	0.880
Base Year 2022 – Post Development	С	32.8	0.901
Future Year 2032 – Pre-Development	D	47.1	0.968
Future Year 2032 – Post Development	D	49.4	0.975

Table 4: Network SIDRA Modelling - Orange Grove Road / Links Avenue - 7.45am - 8.45am

Modelled Peak	Average LOS	Average Delay (sec)	DOS (Veh/C)
Base Year 2022 – Pre-Development	С	33.2	0.900
Base Year 2022 – Post Development	С	36.3	0.919
Future Year 2032 – Pre-Development	F	87.7	1.099
Future Year 2032 – Post Development	F	96.8	1.122

Table 5: Network SIDRA Modelling - Orange Grove Road / Links Avenue - 3.45pm - 4.45pm



#### It can be concluded from the results of the SIDRA modelling that:

- The current operational performance (pre-development) at the cross-intersection of Orange Grove Road / Links Avenue is currently operating at a Level of Service (LOS) ranging between 'A' and 'F' during weekday AM & PM peak.
- The proposed residential development will generally not alter the current LOS at the subject intersection, with the exception of the left turn movement into Links Avenue during the AM peak period which will change from level 'D' to 'E' (a minor increase of 4.1 seconds).
- The vehicle queue length on Links Avenue at the existing intersection will increase from 12.2m to 32.3m (an increase of 3 vehicles) and from 7.9m to 31.5m (an increase of 3 vehicles), during the AM and PM peak periods, respectively. This is acceptable and the post development queue length will be well within the distance between the proposed access driveway and the intersection, and will not impact on vehicular access for the subject site.
- The future 10-year (pre-development) analysis indicates that the subject intersection will operate at a LOS ranging between 'A' and 'F', during weekday AM & PM peak.
- The proposed residential development will generally not alter the future LOS at the subject intersection, with the exception of the right turn movement into Links Avenue during the AM peak period which will change from level 'C' to 'D' (a minor increase in 1.4 seconds).

Therefore, the estimated traffic generation from the proposed development is of low impact on existing flows on Orange Grove Road, Links Avenue and surrounding streets and will not have adverse impacts on the current operational performance of the subject existing intersection, which will generally continue to operate at the same level of service.

The additional traffic generated by the proposed development can be readily accommodated within the existing road layout, without adverse impacts on the amenity of the area.

# 7 RECOMMENDATIONS

The following measures are recommended for the proposed development, to increase traffic and pedestrian safety and reduce the reliance to travel by private cars and vehicle trips, in line with Clause 2.122 *Traffic Generating Development* of the State Environmental Planning Policy (Transport and Infrastructure) 2021.

- A Green Travel Plan be prepared for the subject site, to encourage residents to utilise existing public transport services in the local area and other modes of travel, in order to reduce the reliance on private vehicle trips, where possible.
- Install advisory warning signage at the entrance and throughout the subject site, to advise motorists of a signposted speed limit of 10km/h, 'Slow Down' and 'Watch for pedestrians' signage, to increase traffic and pedestrian safety.
- Install speed cushions where needed, to further reduce the speed environment within the site.
- Install adequate lighting within the subject site, to increase traffic and pedestrian safety.

# 8 CONCLUSION

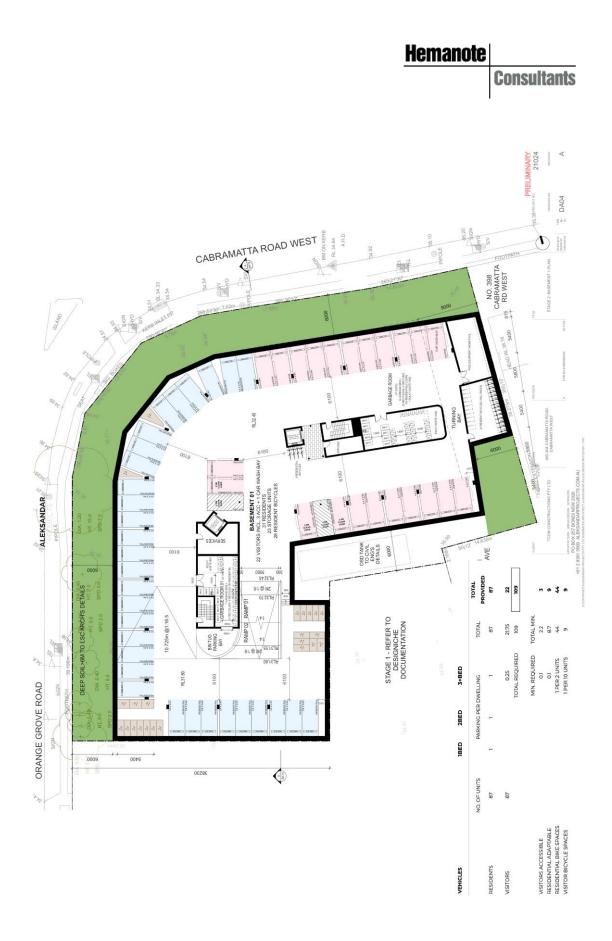
It can be concluded from the traffic and parking impact assessment that the proposed residential flat building and townhouses development at 400-404 Cabramatta Road West, 2-18 Orange Grove Road and 6 Links Avenue, Cabramatta, will not have adverse impacts on existing traffic or parking conditions and is worthy of Council's support in its present form.

- The existing traffic flows on Orange Grove Road and Links Avenue are typical for a state road and a local road, respectively, located in a mixed residential and commercial area, where traffic is well controlled by the existing signalised intersection, with spare capacity.
- The estimated traffic generation from the proposed development is of low impact on existing flows on Orange Grove Road, Links Avenue and surrounding streets and will not have adverse impacts on the current operational performance of the subject existing intersection, which will continue to operate at similar and acceptable levels of service. The traffic generated by the proposed residential development can be readily accommodated within the existing road network.
- The potential increase in the number of vehicle movements in and about Orange Grove Road, Links Avenue and adjacent streets will not have adverse impacts on the amenity of the area.
- The parking demand resulting from the proposed residential development can be easily accommodated within the proposed adequate off-street car and bicycle parking for both residents and visitors, which is in compliance with AS2890.1:2004 and Council's parking requirements.
- The on-site vehicular access, car parking layout and vehicular circulation is adequate for the proposed development and in accordance with AS2890.1:2004, AS2890.6:2009 and AS2890.2:2018, where vehicles enter and exit the site in a forward direction at all times.
- The subject site has good access to existing public transport services.
- The proposed development will not have adverse impacts on parking in the surrounding area.

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Appendix A – Proposed Development Plans (Residential Flat Building)







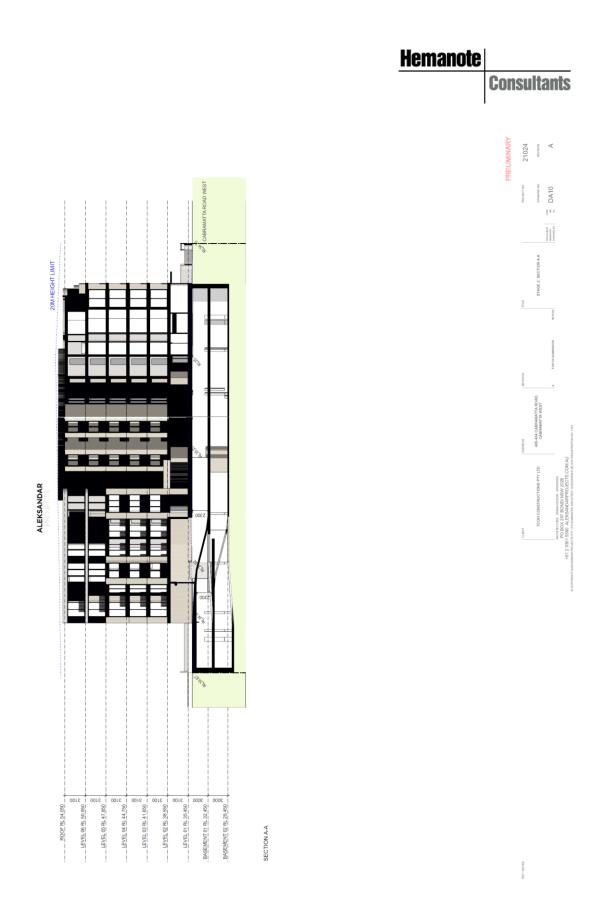


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## Appendix A – Proposed Development Plans (Townhouses)

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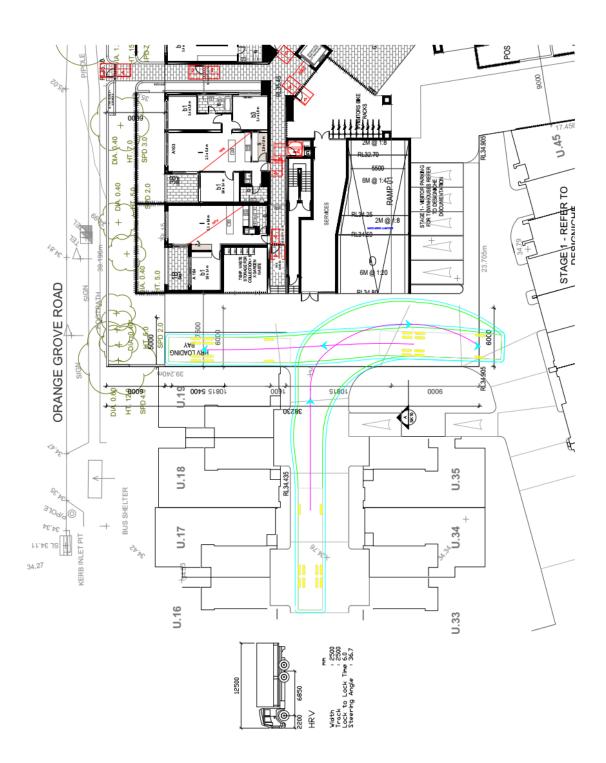
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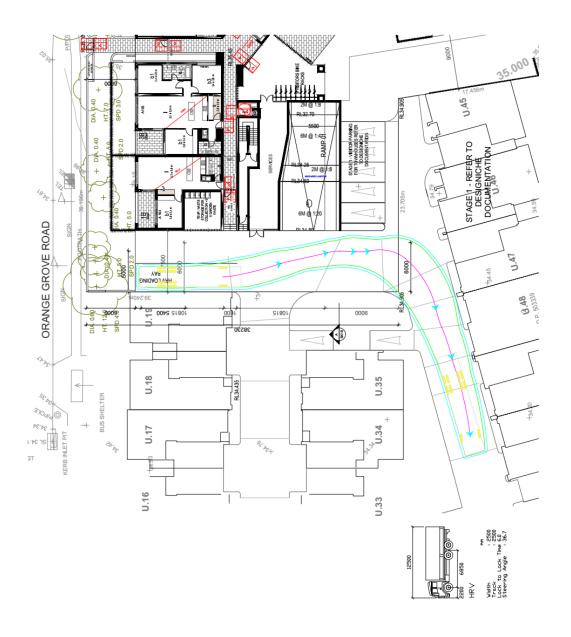
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# Appendix B – Vehicle Swept Paths (Residential Flat Building)

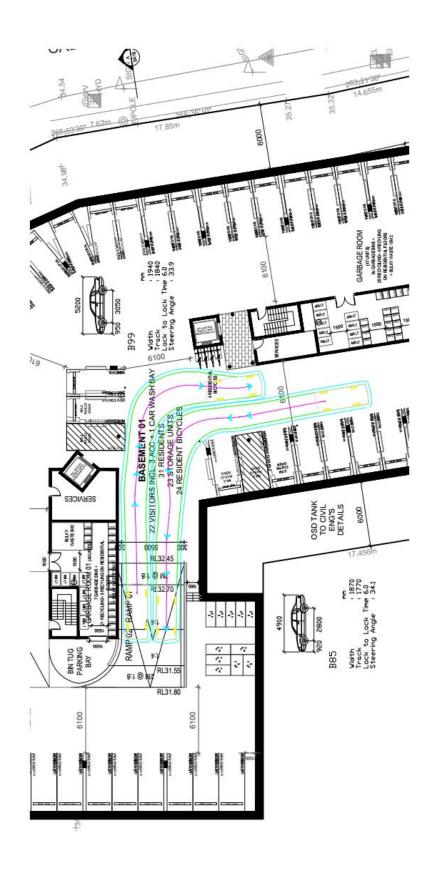
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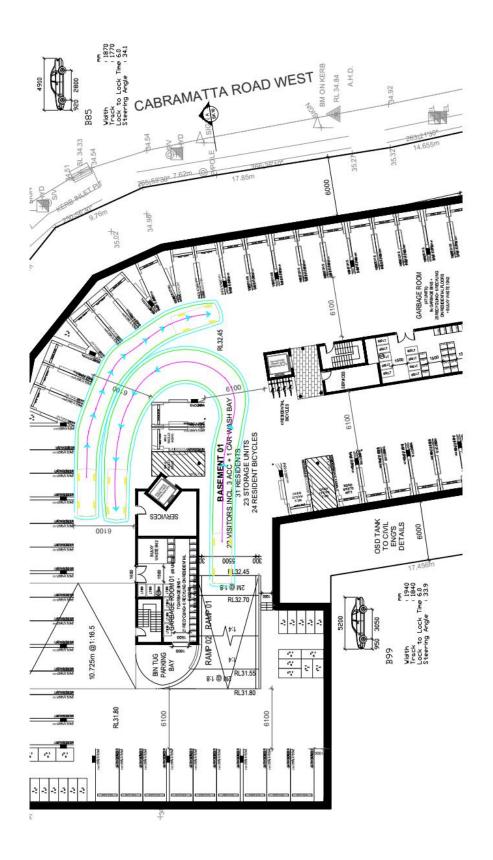
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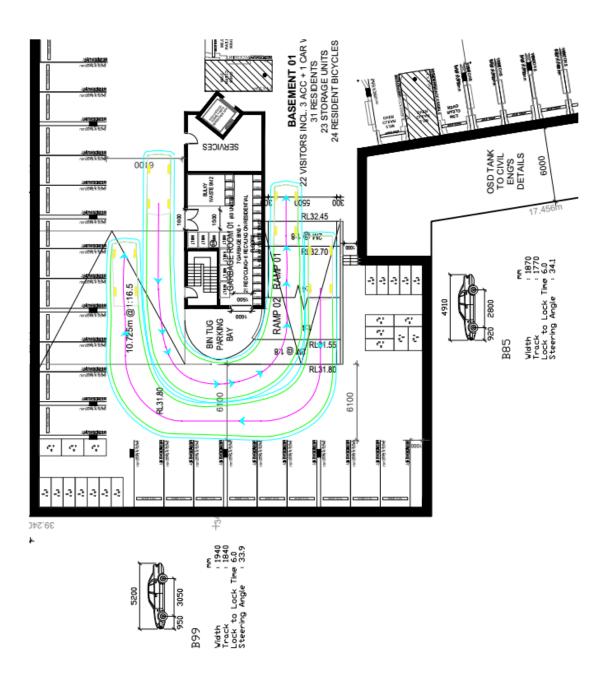


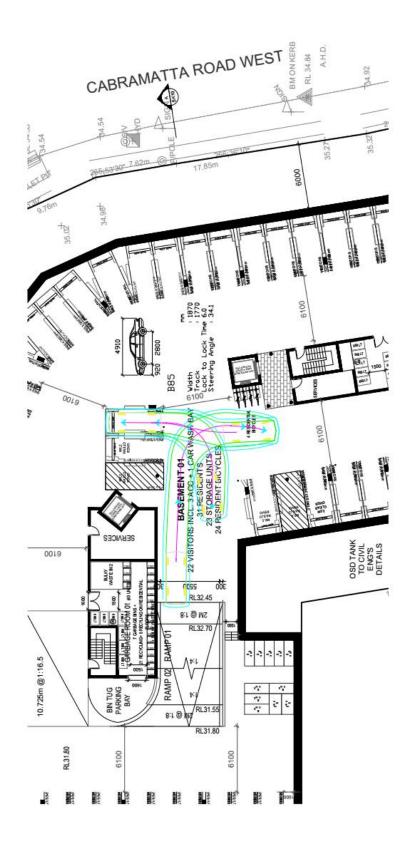
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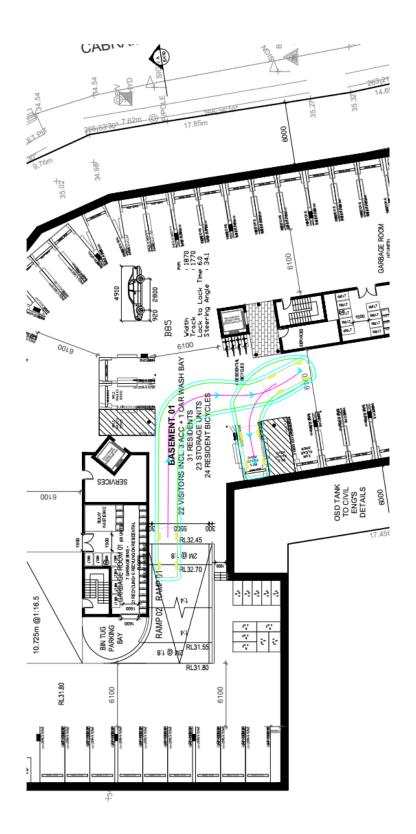


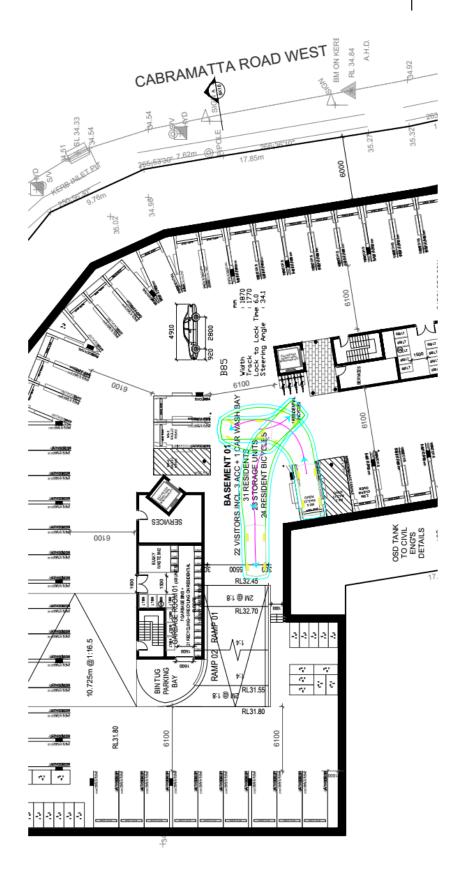
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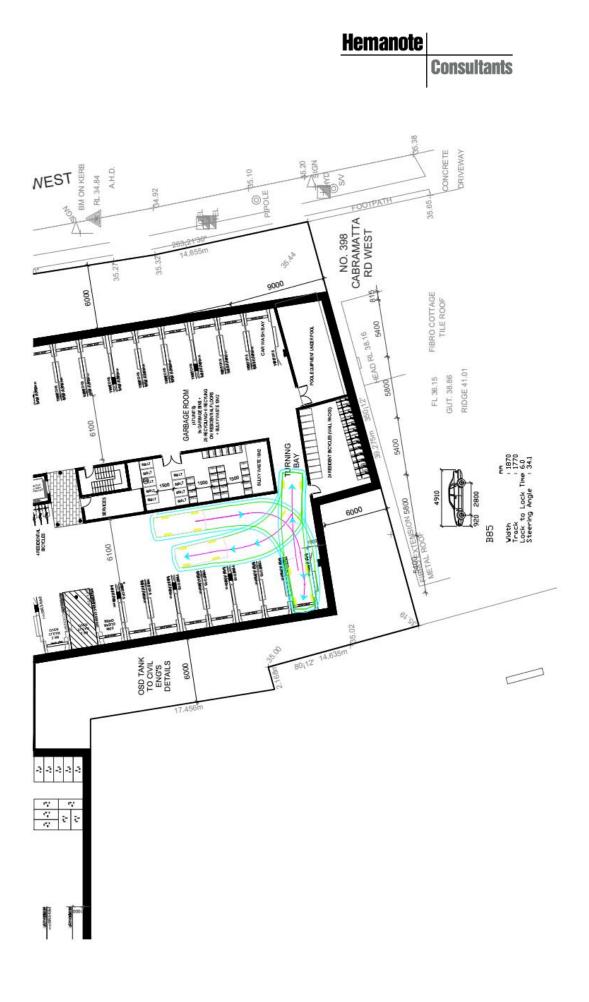
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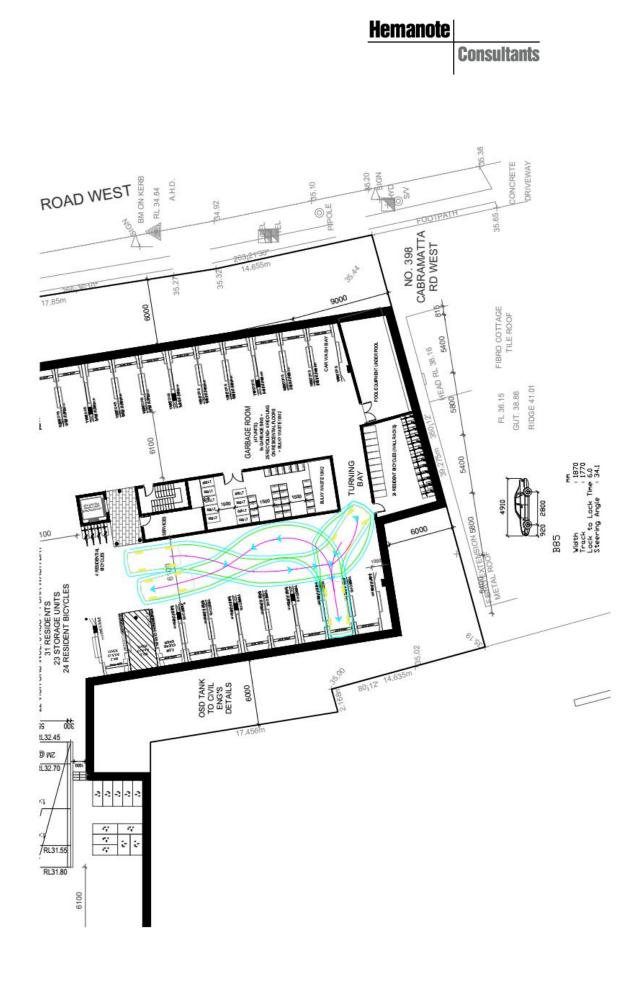


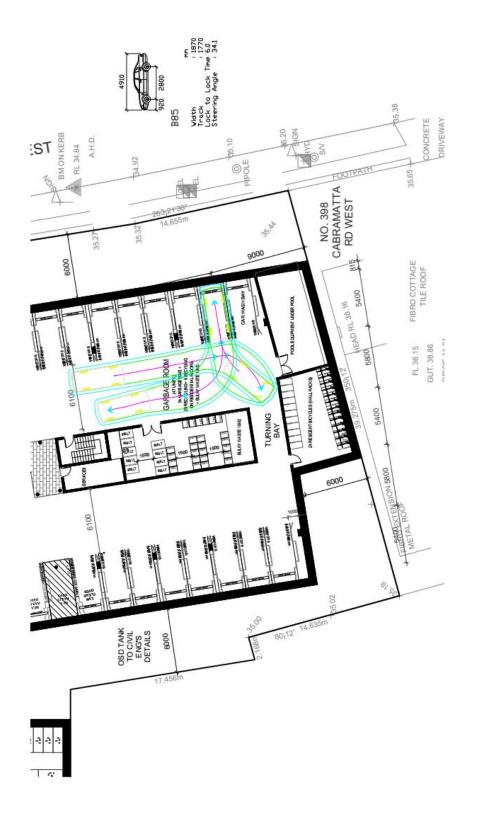


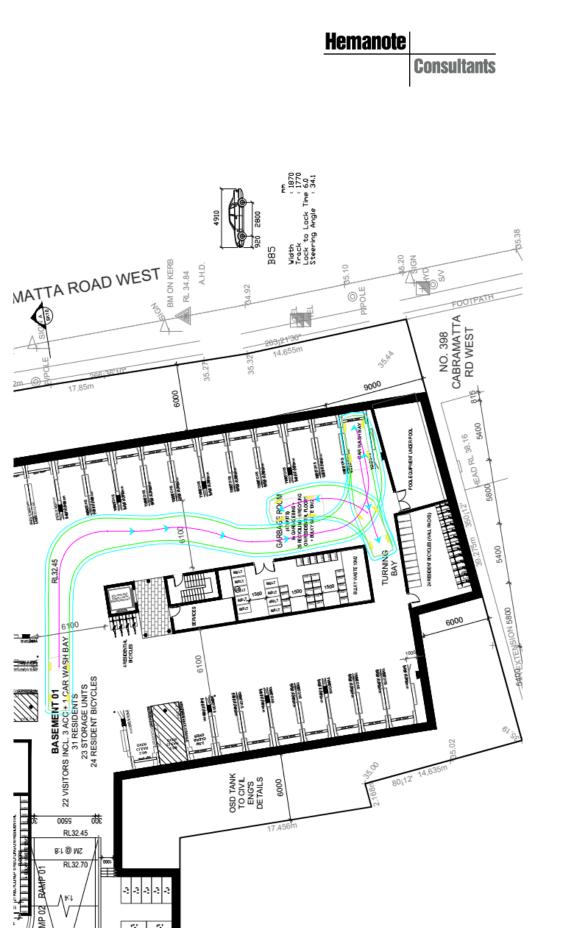




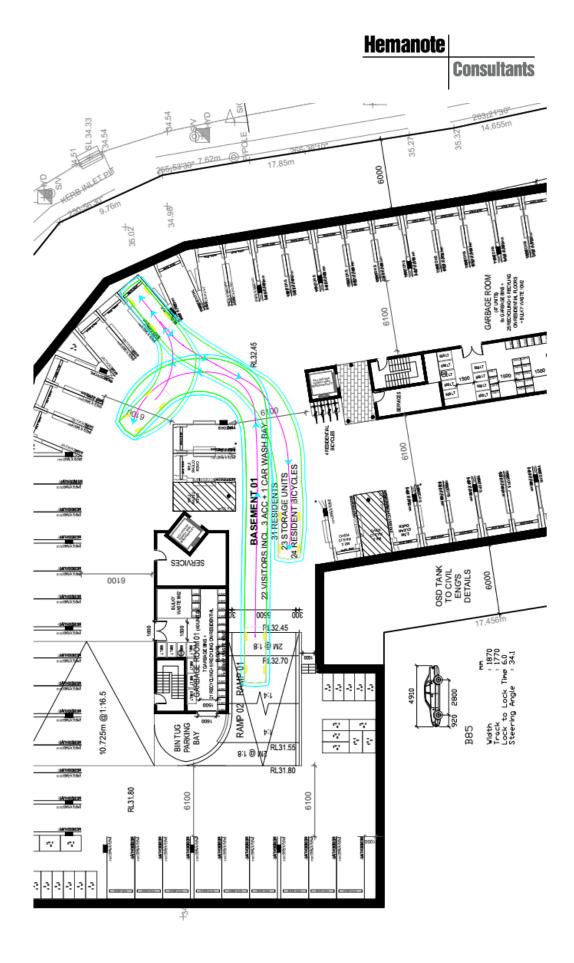








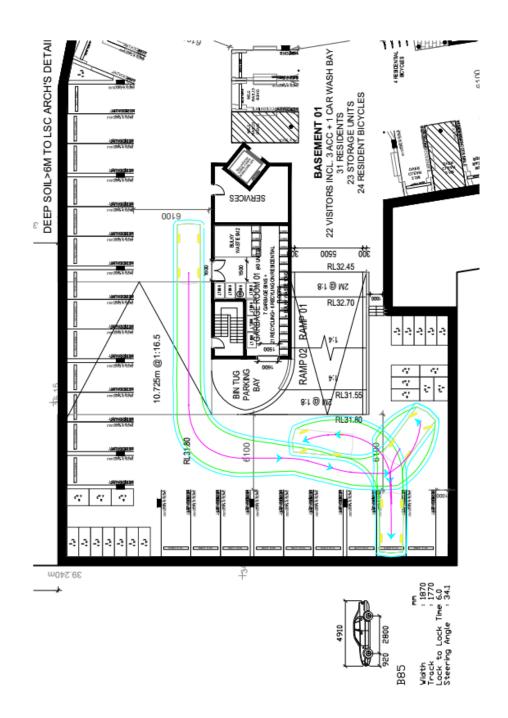
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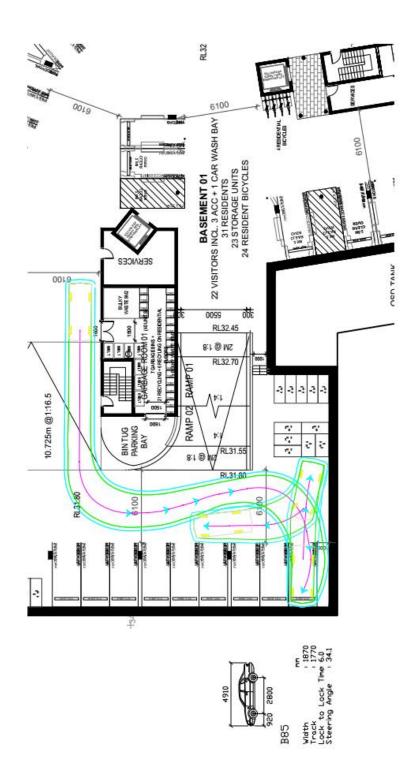
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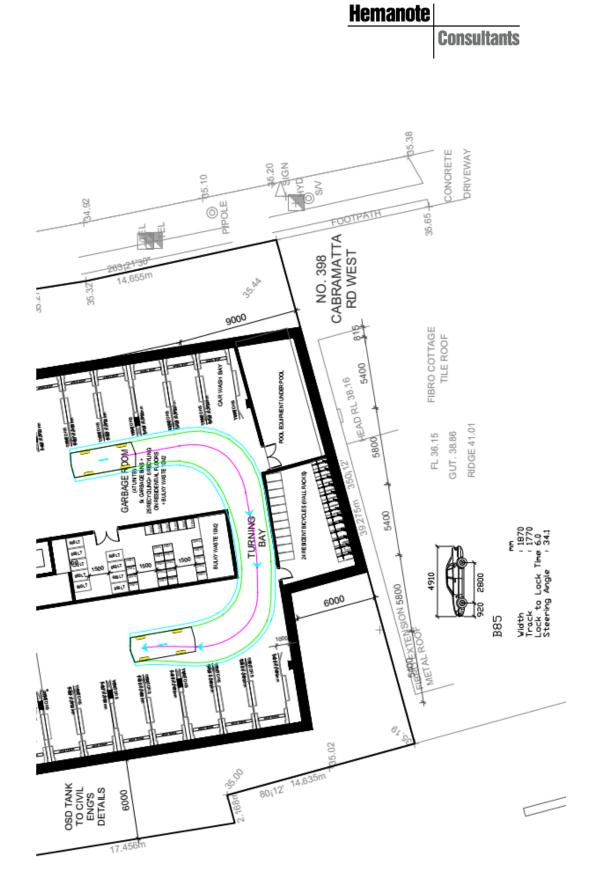


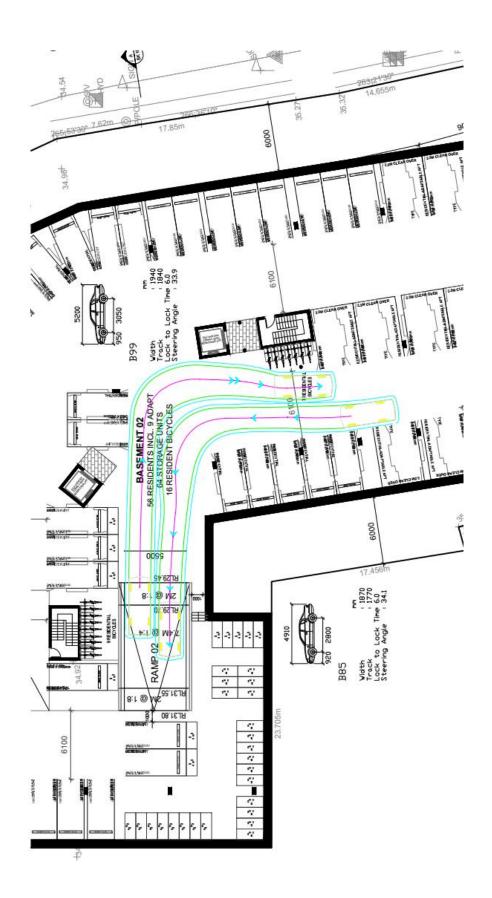
Traffic & Parking Assessment – 400-404 Cabramatta Rd West, Cabramatta 23 May 2023 Page 64



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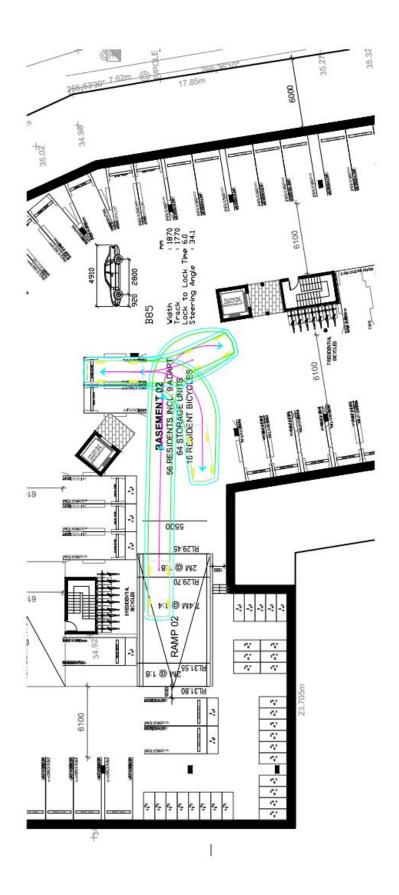
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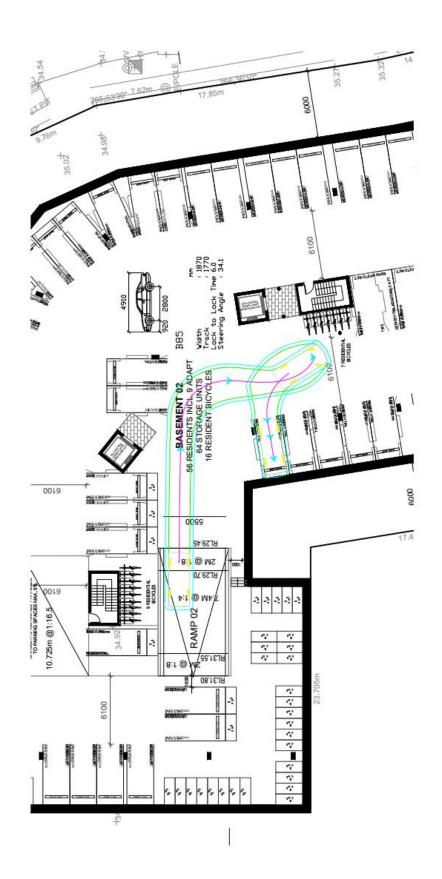


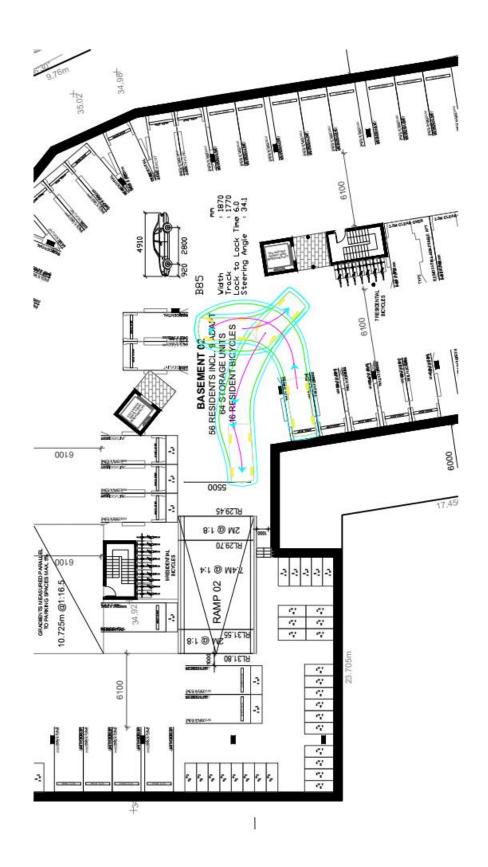


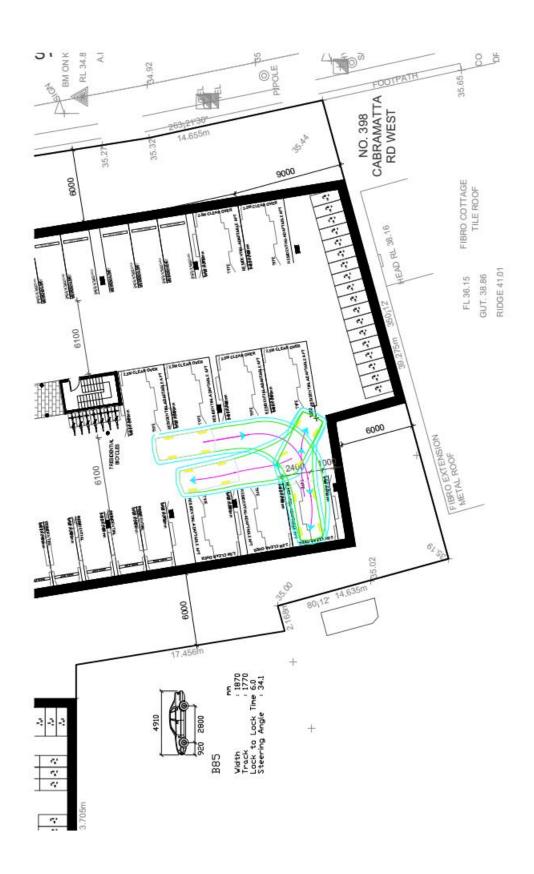
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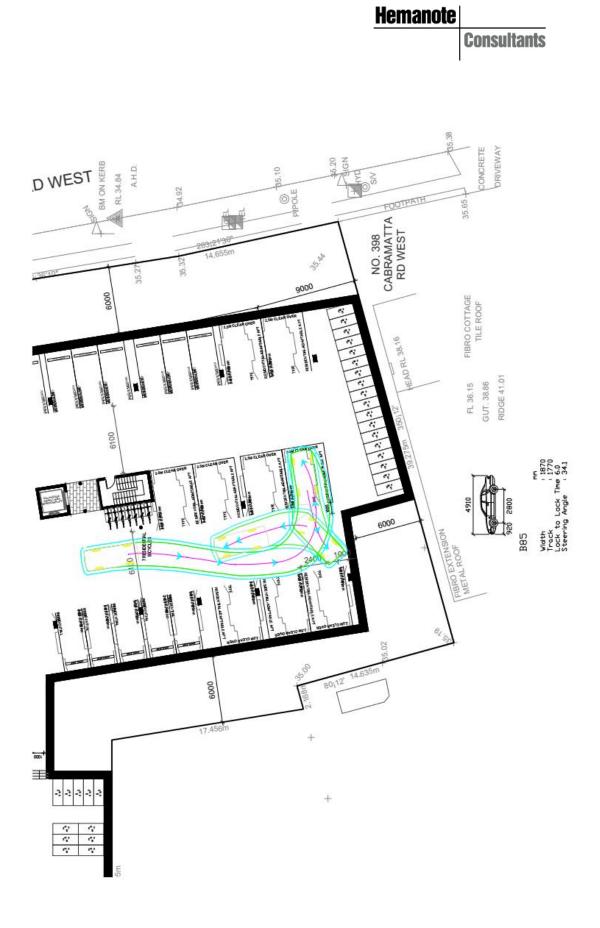








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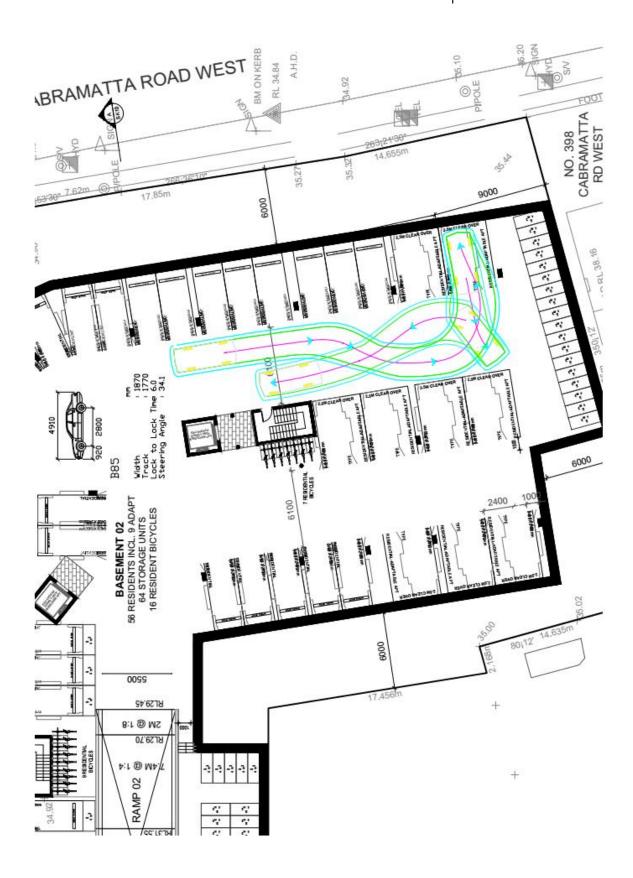
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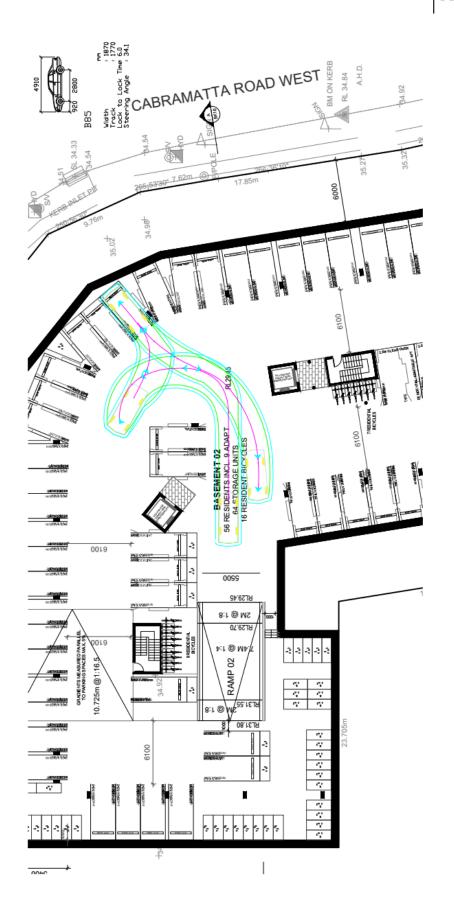
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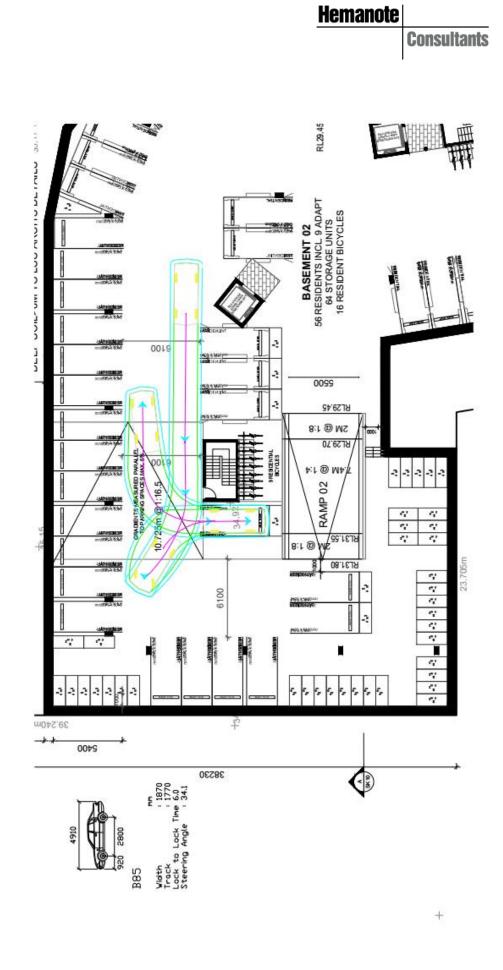
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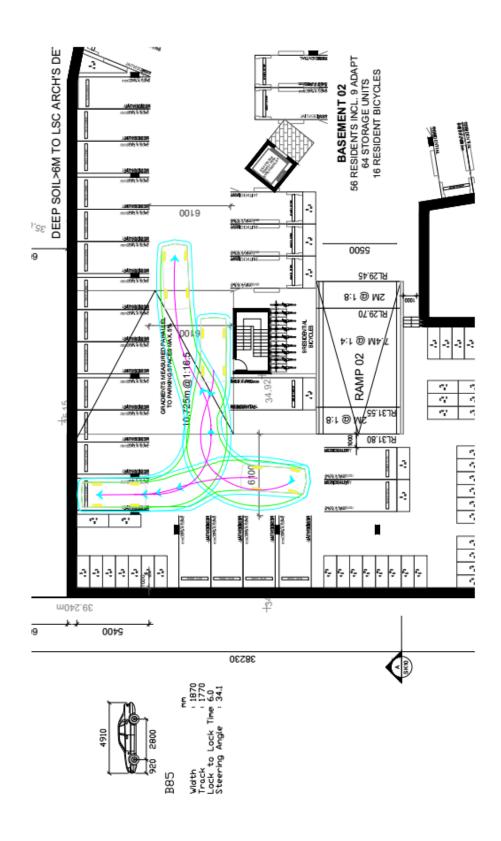
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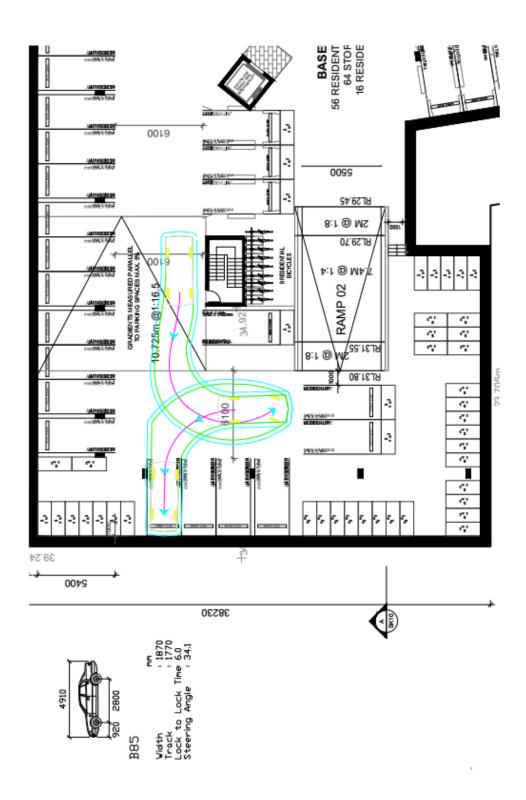
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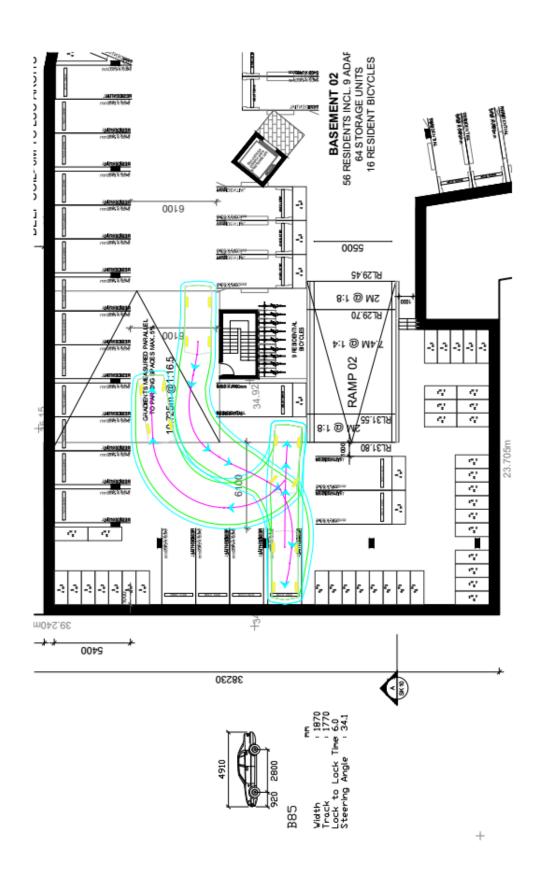






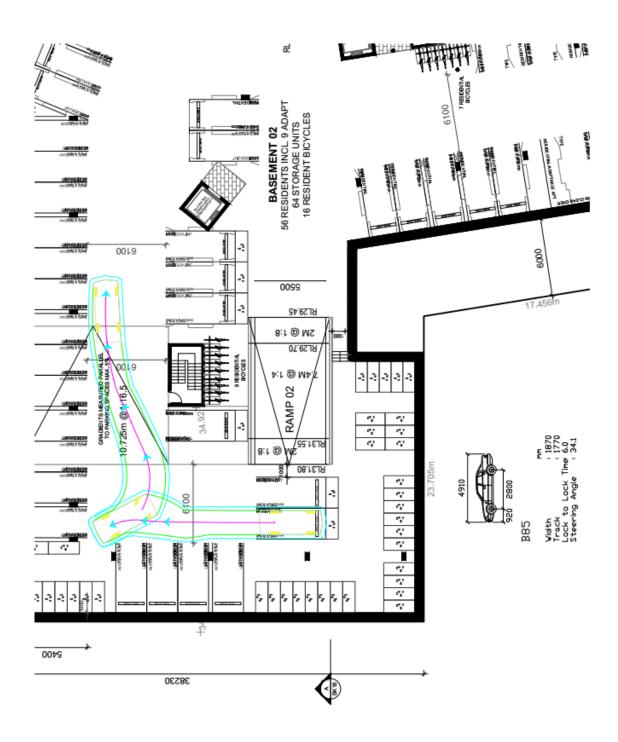




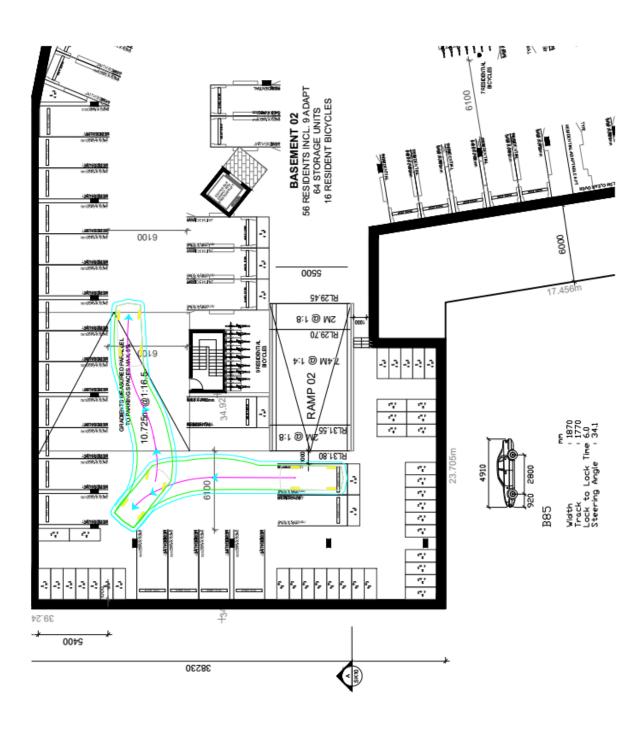


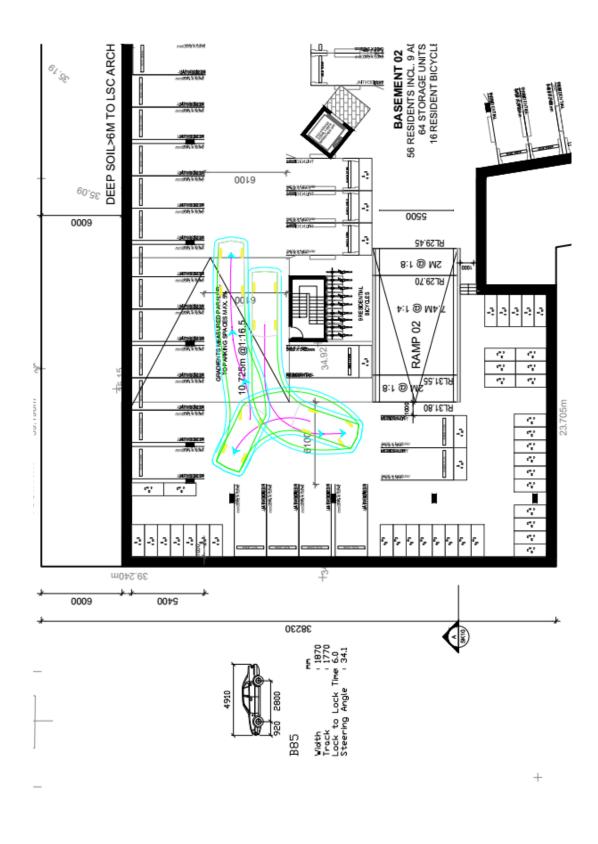
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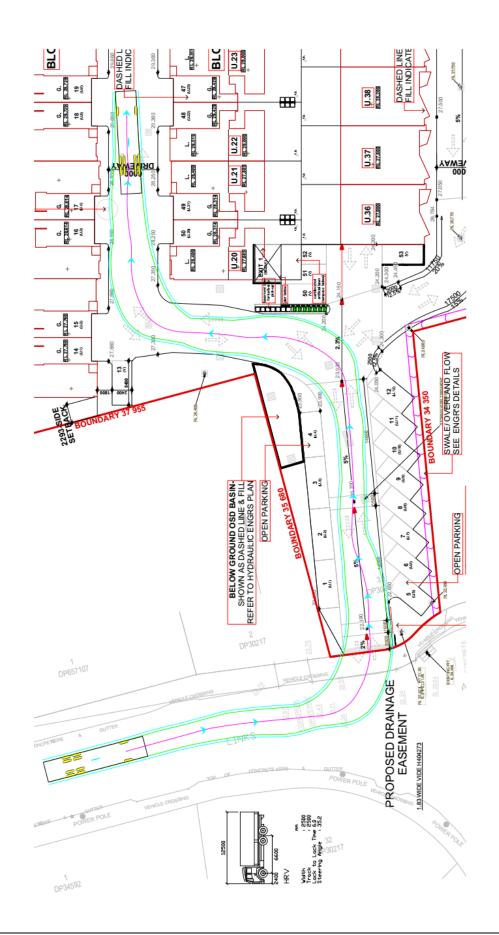
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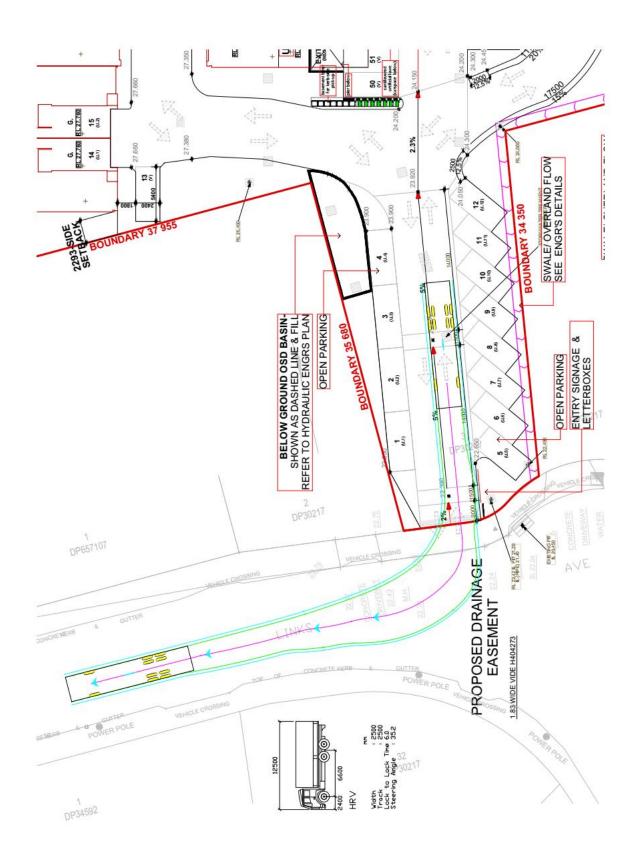


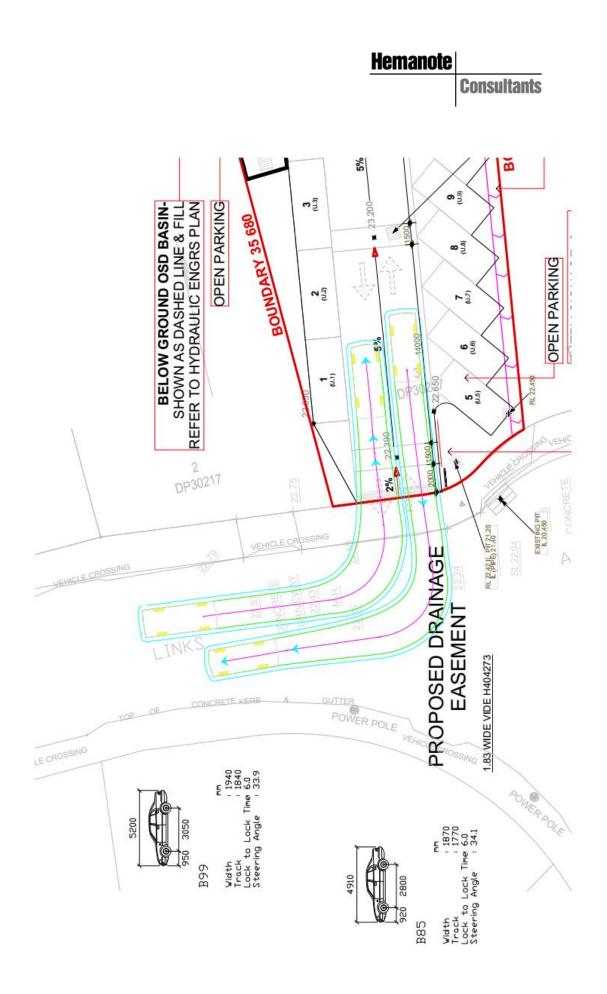


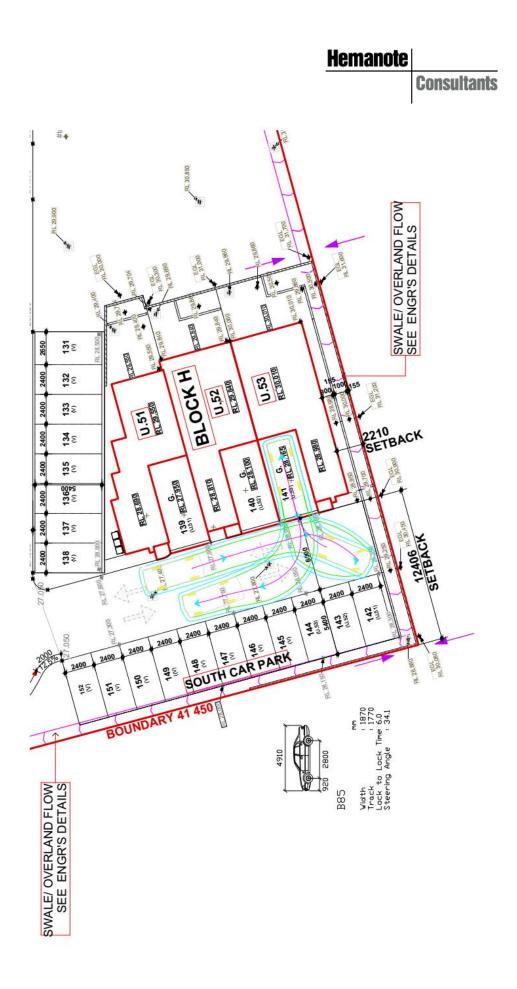
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Appendix B – Vehicle Swept Paths (Townhouses)

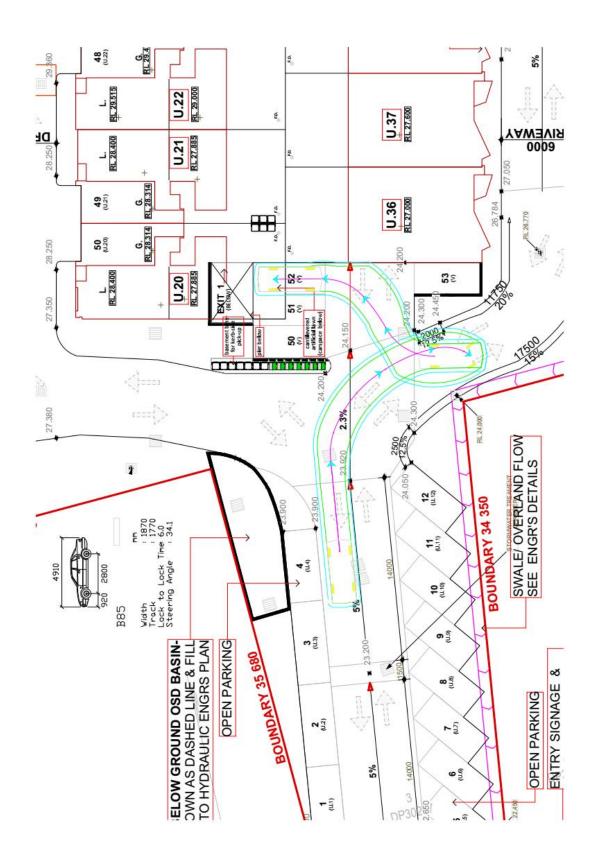






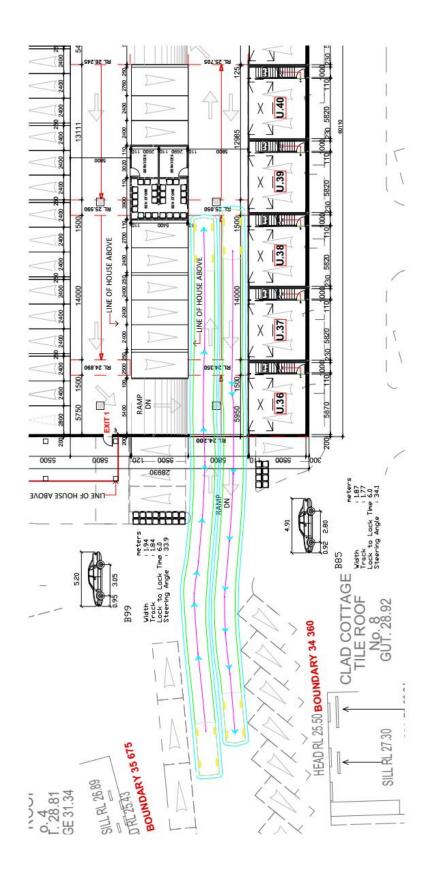


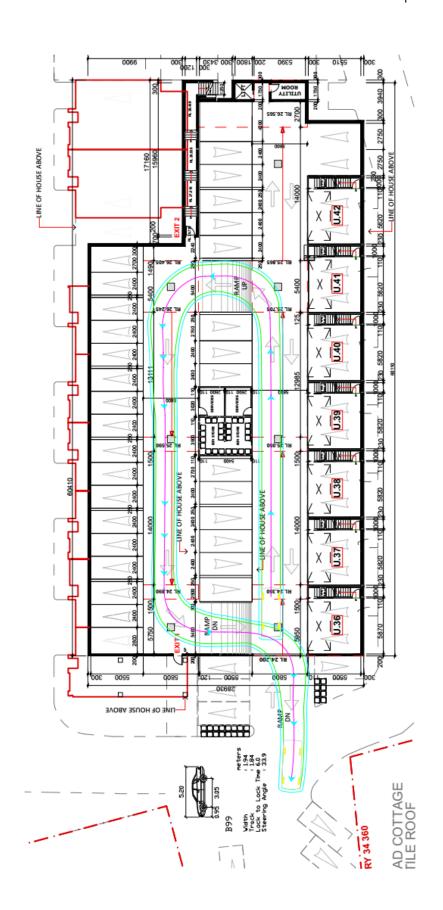
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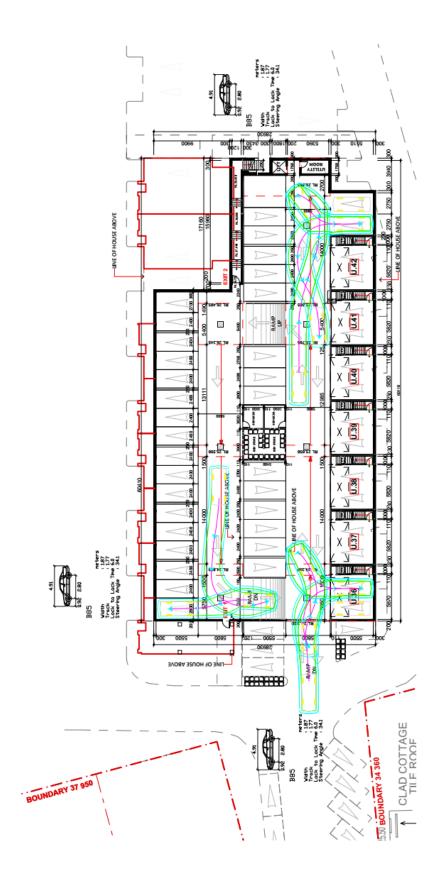


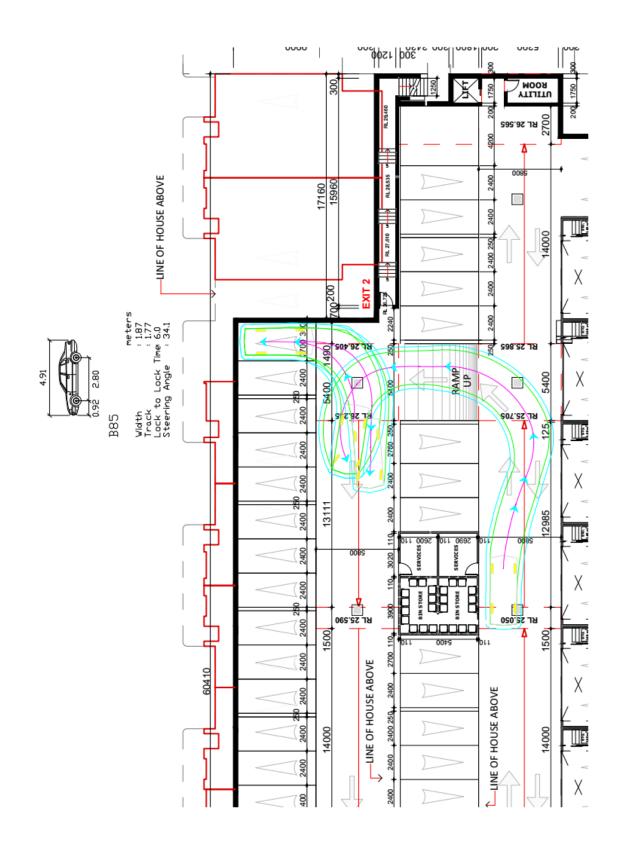
BLOCKG J E 002 RL33.065 U.48 RL 34.261 + 000.<sub>EE</sub> G. 131 BIL 32 896 128 (J.M.) \$<6.20 RL 34.261 + U.49 \$18.20 G. 22.860 (ern) RL 22.125 129 (M.U) RL 2255 OPEN PARKING 32.800 RL 33.586 + U.50 106 (U.50) 008.25 0<sub>59</sub>.25 3**105** G. 590 130 (150) 164 **5**5 RL 33.586 L'SZ 5032 13500 6933 SEPARATION RL 32.960 U.44 mm 1770 6.0 34.1 RL 33.126 Time **BLOCK F** Vidth Track Lock to Lock T Steering Angle 4910 2800 **RL 32.800** (0 25 B85 + RL 33.126 103 (1.44) G. G. G. <u>Rt. 31.680</u> 102 103 (U.44) (U.44) 31.135 8 + 31.15

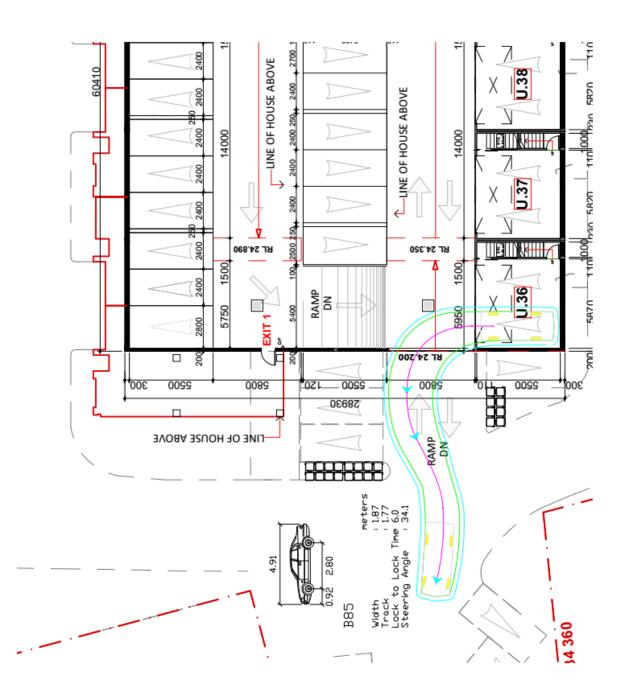
0 -RL34 33.000 G. BU 128 32,895 RL 34.261 U.49 RL 32.860 101 ó SL 327 25 129 OPEN PARKING RL 32265 32.800 RL 33.586 + U.50 106 (U.SO) -86 3105 RL 20.000 130 **104** RL 33.586 Segs 13500 6933 SEPARATION FL 32.960 тт 1870 1770 8.6.0 34.1 U.44 Width 11 Track 11 Lock to Lock Time 6 Steering Angle 13 RL 33.126 4910 **BLOCK F** 2800 RL 32.800 920 B85 31 385 + RL 33.126 103 (U.44) G. G. G. RL 31.680 102 103 (U.44) (U.44) RL 30.870 RL 31.050 30.900 RL 31 215 B 005 - 6

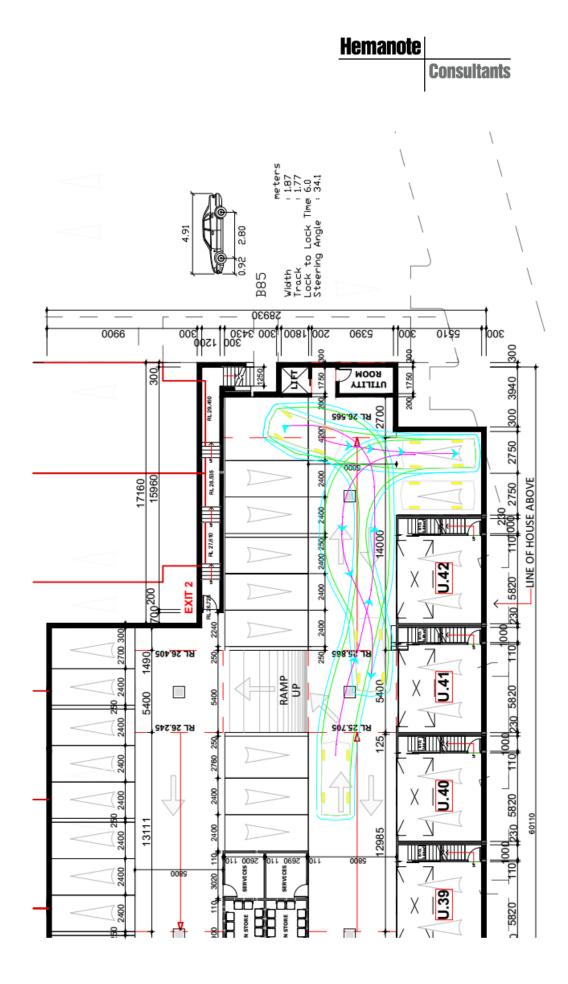


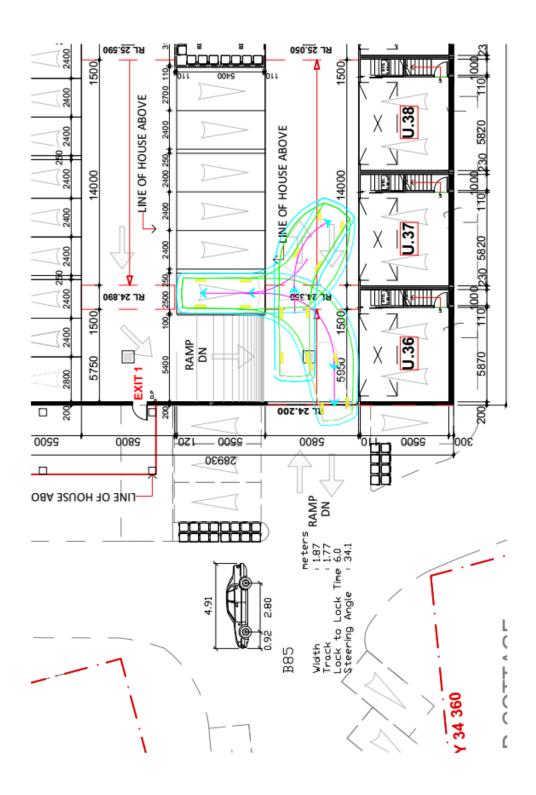


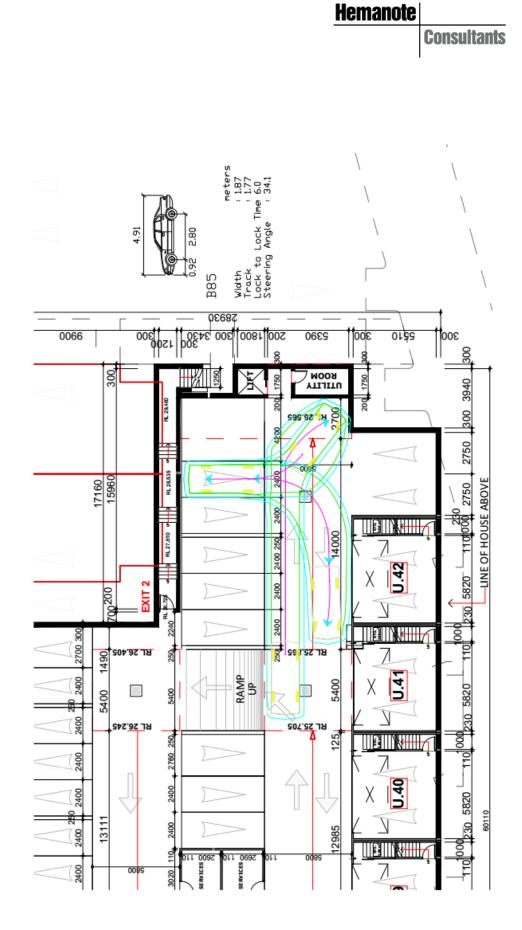






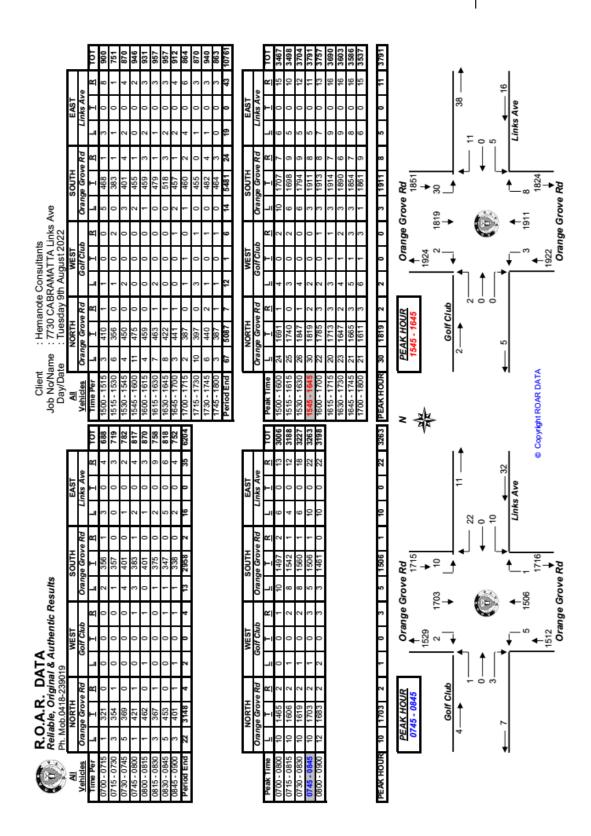


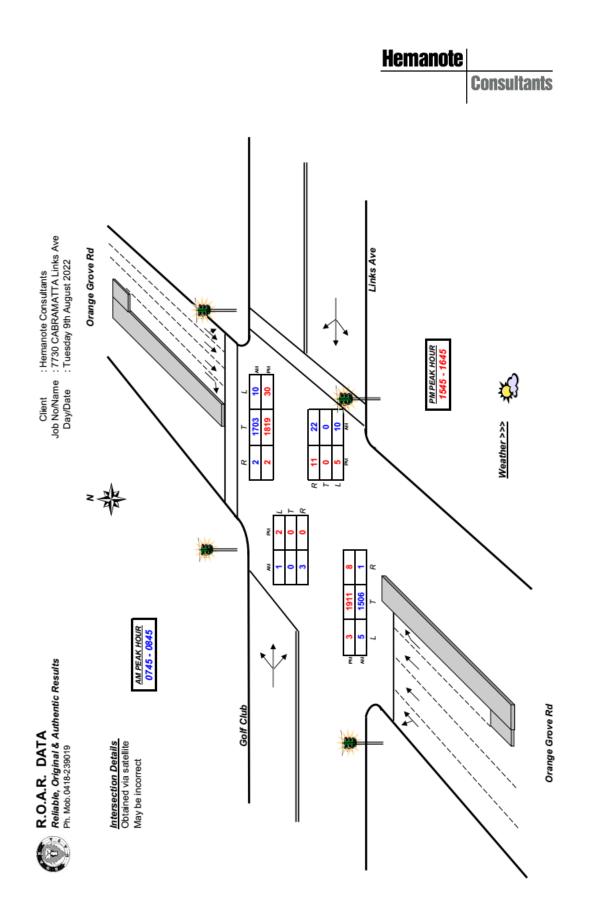




Hemanote	
	<b>Consultants</b>

## Appendix C – Traffic Volume Counts





Consultants

# Appendix D – SIDRA Intersection Results

**Consultants** 

MOVEMENT SUMMARY – 7.45am – 8.45am – Base Year 2022 – Orange Grove Road / Links Avenue

Vehicle Movement Performance Mov Tam INUT DE VOLUNAL 11 VOLUNAL 101	wernent Perfo Notuber Total HVT	Perform 1855 1871	DEMAND DEMAND FLOWS (Total IN)		25 f	Aver Level of Delay Service	100550	SAN BACK OF OLDUE [Viet. Dist]	žð	28 B	툍 <i>됕</i>	Annel Special	Vehicle Nov Tu ID	unom n ≥ <u>n</u>	rehicie Movement Performanca 66 Tam R-VII DE 1744 HVI [Tau	Ormans [] []	DEMNED DEMNED Technol Technol	5	Aver Level of Delay Servers		ISS BACK OF QUEIE [Web Dat]		Prop. Effective Oue Step Rote	₹±8	
South: Onange Grove Avenue	te Grove	Avenue											South: 0	Orange 6	Orange Grove Avenu	300									
31	9	0.0	5	00	0.494	15.3 LOSB	122	90.2	10.64	0.56	190	42.7		5 27	00 .00	5	000	0.494	153	LOS B	12.2 9	90.2 0	0.64 0.56	8 064	4 42.7
- F2	- 10	00	- 10		0.002	364 LOSC		03	120	0.61		37.4	- 22					0.020	696	080					
Approach	1512	7.0	1592		0.494	9.0 LOSA		903	0.64	0.55		58.7	Approac					0.454	92	08 A					
East Links A	Avenue												East Links	nis Avenu	000										
4	₽	00			0.125	500 LOSD		12.2	0.88	0.72	0.88	31.8	4					0.318	21	080					
6 TI	- 8	00	- 8	* 00	0.125	45.5 LOSD	11	122	0.88	220	0.89	14.3	5 11	- 20	000	- 5	00	* 0.318	475	1080	46 3	0 1 0	0.92 0.77	7 0.02	0 212
Approach	8	0.0			0.125	49.9 LOS D		122	0.88	0.72	0.68	26.2	Approact					0.318	62.0	080					
North: Orange Grow		Averue											North: O	North: Orange Grove	Arren Aven	8									
7 12	10	0.0						278.2	1.00	1.01		22.5	7 1					0.901	58.4						
# G	5071 c	01	1793	7.0 +0	0.880	48.0 LOS D	32.6	278.4	1,00	101	1.15	7.16	8 9	11 1703	01 00	1793	3 7.0	+0.901	620	1080	40.3 24	298.9 1	1.00 1.04	M 1.19	9 332
Approach	1715	1.0						278.4	1.00	1.01		21.6	Approact				Ľ.	0.901	622						
West Golf CI	Club Privat	ate Road											West G	Golf Club Pro	Private Ros	190									
10 L2	-	0.0	-			472 LOSD		1.8	0.85	0.63		16.0	10					0.019	47.3	080					
11 11	-	0.0	-	0.0	0.019	44.0 LOS D	0.3	1.8	0.85	0.63	0.85	17.5	11 11	1 11	8	-	0.0	0.019	44.1	1080	0.3	1.8 0	0.85 0.63	3 0.85	6 17.6
12 R2	e	00				473 LOSD		18	0.85	669		0.05	12 R.					0.019	473	080					
Approach	w.	0.0				46.6 LOS D		1.8	0.85	063		25.6	Approact					0.019	46.6	080					
All Vehicles	3265	6.9	3437	69	0.880	30.0 LOS C	37.5	278.4	0.83	0.60	0.91	42.6	All	3367	67 67	3544	4 67	0.301	32.8	LOS C	40.3 25	296.9 0	0.83 0.82	Q 0.93	6 40.9

MOVEMENT SUMMARY – 3.45pm – 4.45pm – Base Year 2022 – Orange Grove Road / Links Avenue

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Vehicle Movement Performance Mov Tum Aurus Edit D Tala Pov (Total	NET Perfor NRUT VOLINES	DE DE DE DE DE DE DE DE DE	mance DEMAND FLOWS FLOWS	Zā (	Anter Lavel of Datay Service		I WE DAT	₹ð.	and a second	All and a state of the second	Aver Areas	New Classification of the second seco	Turn T	Veholis Movement Performance New Tam ArPUT DE No. 1944 N. 1944 New 1974 ArV		DEMAND FLOWS M	21 ×	i Anar Lanal of Distay Service		SS BACK OF LINE OLD IN LINE DIAL	뢅쾽	58.D	1 = §	11 1
South Orange	Grove Avenu	8			100	100						Sout	th: Orang	+ Grove A	entre									
1 12	00 8			0 557	151 1.051				054	09.0	42.9	-	a	6			0.557	15.1				0.54	0.60	42.9
2 11 19	11 7.0			*0.557	87 108				054	090	59.1	71	F	1161				8.7				0.54	0.60	59.1
3 R2 8 0.0	8 0.0	8	0.0	0.013	40.7 LOS C	0.4	2.8	0.68	1910	990	36.7		8	3 R2 18 00		19 0.0		41.0		0.9 6.3	0.69	690	6910	36.6
Approach 15	02 20			195.0	88 1061				0.57	090	6.85	466	hoadh	1932			0.667	30	LOSA 18			0.54	0.60	299
East Links Aver	976											East	Links /	Series .										
			00	0.074	0		92	0.00	0.60	0.00	1 01	1						6.69	LOSE 41				050	27.7
32			0.0	1000	3 5			0.00	000	0.00	6.03	IJ	Ŧ				1	61.4					0.93	11.8
				0.074	\$ 5		0	0.00	800	0.89	18.8	9	6 R2	12	0.0	42 0.0	0.283	6.89	LOSE 45	5 315	0.30	0.76	050	10.3
Approach 17	17 0.0	85	00	0.074	63.0 LOS	1	61	680	690	68.0	21.7	App	roach					659					0:00	21.3
North: Orange	Stove Avenu											Nort	North Orange	e Grove Av	Avenue									
2 13 2	0. 00			0.000	1001 202				1011	1 + 2	50.0	7					616.0	E.69	LOSE 53			1.03	1.17	18.8
14	10 20			0.000	201 100				-	1	D 40	8						62.9				1.03	1.17	29.9
	00 00			acuu-	100 100 100 B				190	1000	C 10	m	9 R2	2	0.0	2 0.0	820.0 = 0	83.6	LOSF 0.2	2 1.1	0.98	0.61	B6:0	103
Approach 1051 6.9	69 19	1948	69	0.900	582 LOSE	E 50.4	373.9	1.00	101	1.13	312	App						63.2				1.03	1.17	29.6
West Goff Chile	Private Ros											Wes	West Golf Club Priva	db Private	R									
					1							9	2										0.87	13.3
2	en z			0.016	3		2	180	79/0	180	13.3	=	E					515					0.87	14.4
	00		00	0.016	200		2 :	180	290	18.0	14.4	12	R2		0.0	1 0.0	0.016	109	LOSE 03	3 1.8	0.87	0.62	0.87	26.0
Aermach 4 0.0	00 1	- 4	a a	0.016	494 1081	E 03	1.8	0.07	200	0.07	17.4	App	hoadh	1000				6.65					0.87	17.4
internation of the					8					-		N		1000	57 6	600C CT	r 0.640	1 M	IDSC 518	1002 10	nsn 1	0.78	0.68	C BL
All 37 Vehicles	6.9 MS/E	MAGE	69	0050	332 L050	C 50.4	3739	61.0	0.77	0.86	40.9	Viela	Vieledes					R					8	N.



Whicke Movement Performance New Target State (1975, BACK) First Effective And New Target State (1971, 1972, 1974) Same State State (1974, 1971, 1974, 1971, 1974, 197	arge Grove Avenue	5 0.0 6 0.0 0.533 15.2 LOSB 17.1 126.8 0.59 0.53 1506 7.0 1902 7.0 ±0.533 8.9 LOSA 17.1 126.9 0.59 0.53	3 R2 11 0.0 14 0.0 0.024 4.36 0.080 <sup>11</sup> 0.7 4.8 0.72 0.68 0.72 Amounts 1673 6.0 1031 6.0 0.651 0.1 1062 0.71 1360 0.00 0.61 0.01	tone we take up and at board it's take was very	ADDRESS A ADDRESS ADDRESS ADDRE		58 0.0 73 0.0 84 0.0 105 0.0	Avenue	0.0 64 0.0 0.975 89.7 LOSE <sup>1</sup> 69.9 516.0 1.00 1.14	8 Ti 1703 7.0 2151 7.0 +0.975 834 (08.F) 69.9 518.8 1.00 1.15 1.30 9 R2 2 0.0 3 0.0 +0.034 837 (08.F <sup>3</sup> ) 0.2 1.3 0.99 0.62 0.99	6.8 2218 6.8 0.975 835 (08.F <sup>1</sup> 69.9 518.8 1.00 1.15	Club Prinate Road	1 0.0 1 0.0 0.026 61.1 LOSE 0.4 2.7 0.87 0.54	00	5 00 6 00 0056 605 LOSE <sup>4</sup> 04 27 087 064	All 3597 6.7 4253 6.7 0.975 49.4 <mark>(05.0</mark> ° 60.9 518.8 0.81 0.66 0.97 Venides
t Effection Aven Aven Bage Mo Speed	Brraft .	650 650	0.02 0.70 35.0	650 650		060 670	7.81 0.60 670 0	1. Sec. 1.	114 129	0 1.14 1.29 26.9	1.14 1.29		0.64 0.87	0.64 0.87	7 0.64 0.87 22.0	1 0.65 0.96 34.9
165 RACK OF Prop. Effective SOLELLE Que Bace 1 Vet. Date 1	-	LOS 0 059 059 059 059 059	LOSC 0.1 0.4 0.70 0.62 0.70	LOSA 17.1 126.7 0.59 0.53 0.59		LOSE 27 18.6 0.90 0.73 0.90 LOSE 27 18.6 0.90 0.73 0.90	LOSE 27 186 0.90 0.73 0.90 LOSE 27 186 0.90 0.73 0.90		LOGF 66.9 495.8 1:00 1.14 1.29	LOSF 669 496.4 1.00 1.14 1.29	LOSF 669 496.4 1.00 1.14 1.29		LOSE 0.4 2.7 0.87 0.64 0.87	LOSE 0.4 2.7 0.87 0.64 0.87	USE 04 27 087 054 087	<mark>1030</mark> " 669 496.4 0.81 0.85 0.96
Prop. Effection Que Bage Rate	wheth N at the state of	6 00 0533 152 LOBB 171 1266 059 053 059	424 LOS C 0.1 0.4 0.70 0.62 0.70	1910 7.0 0.533 89 LOSA 17.1 126.7 0.59 0.53 0.59		13 0.0 0.165 63.6 LOSE 2.7 18.6 0.90 0.73 0.90 1 0.0 0.165 59.0 LOSE 2.7 18.6 0.90 0.73 0.90	27 186 0.90 0.73 0.90 27 186 0.90 0.73 0.90		0.0 13 0.0 0.968 86.8 LCGEF 66.9 495.8 1.00 1.14 1.29	70 2151 70 -0.968 004 LOGF 669 4964 1.00 1.14 1.29	2166 7.0 0.960 80.4 LOSF <sup>1</sup> 66.9 496.4 1.00 1.14 1.29	Club Phrate Road	1 0.0 0.025 60.1 UOSE 0.4 2.7 0.87 0.64 0.87	1 0.0 0.025 56.9 LOSE 0.4 2.7 0.87 0.64 0.87	0.4 2.7 0.67 0.64 0.87	669 4%4 0.81 0.85 0%



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Willing Textual (MA)	Vehicle Movement Perfor	mance										Vehicle Movement Performance							
optimization South Charae C	Mos Tem INDUT D VOLUMES (Total HV) with N	8~콜	88 <sup>5</sup>	and the second se	19 x 19 x		A BACK			124	II I	Nov All the second seco	ALC: No.		URA DATA		Prop. Effective One 1999 Fore	N S S	11 1
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America <t< td=""><td>1922</td><td></td><td>2.0</td><td>0.676</td><td>30.4 LC</td><td></td><td></td><td></td><td></td><td></td><td></td><td>1932 6.9 2440</td><td>10.6</td><td></td><td></td><td></td><td></td><td></td><td></td></t<>	1922		2.0	0.676	30.4 LC							1932 6.9 2440	10.6						
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161 63 103 63 103 103 103 103 103 103 103 103 112 112 103 103 103 112 103 103 103 112 103 103 103 103 112 103 103 103 112 103 103 103 103 103 103 103 103 103 103 103 103 103 208 103 203	2		0.0	+0.034								2 0.0 3	83.7						
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