

**400-404 CABRAMATTA ROAD WEST, 2-18 ORANGE
GROVE ROAD & 6 LINKS AVENUE, CABRAMATTA**
PROPOSED RESIDENTIAL FLAT BUILDING AND TOWNHOUSES

UPDATED TRAFFIC & PARKING IMPACT ASSESSMENT

MAY 2023

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400-404 CABRAMATTA ROAD WEST, 2-18
ORANGE GROVE ROAD & 6 LINKS AVENUE,
CABRAMATTA
PROPOSED RESIDENTIAL FLAT BUILDING AND
TOWNHOUSES
DATE: 23 MAY 2023

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1 INTRODUCTION

This report has been prepared by Hemanote Consultants to assess the traffic and parking implications of the proposed residential flat building and townhouses development to be located at **400-404 Cabramatta Road West, 2-18 Orange Grove Road and 6 Links Avenue, Cabramatta**, accommodating eighty-seven (87) residential apartments over two basement parking levels and fifty-three (53) townhouses with at-grade and basement parking.

This report is to be read in conjunction with the architectural plans prepared by Alexandar Projects and Designiche Building Designers (reduced copy of the architectural plans are attached in Appendix 'A' of this report) and submitted to Fairfield City Council as part of a Development Application.

This report is set as follows:

- *Section 2:* Description of the existing site location and its use;
- *Section 3:* Description of existing traffic conditions near the subject site;
- *Section 4:* Description of the proposal, vehicular access, on-site parking provision, layout and circulation;
- *Section 5:* Assessment of impacts on parking;
- *Section 6:* Assessment of impacts on traffic in the vicinity of the subject site; and
- *Section 7:* Outlines conclusions.

2 EXISTING SITE DESCRIPTION

➤ Site Location

The subject site is located on the southern side of Cabramatta Road West at properties No. 400, 402, 402A and 404 (legally known as Lot 1 of DP29449, Lots 1 and 2 of DP503339, and Lot 6 of DP709126), on the eastern side Orange Grove Road at properties No. 2-18 (legally known as Lot 7 of DP709126), and on the northern side of Links Avenue at property No. 6 (legally known as Lot 3 of DP30217), within the suburb of Cabramatta. The site has a primary frontage of 49.9 metres to Cabramatta Road West from the north, a secondary frontage of approximately 185.8 metres to Orange Grove Road from the west and a third frontage of 14.9 metres to Links Avenue from the south. Refer to Figure 1 for a site locality map.

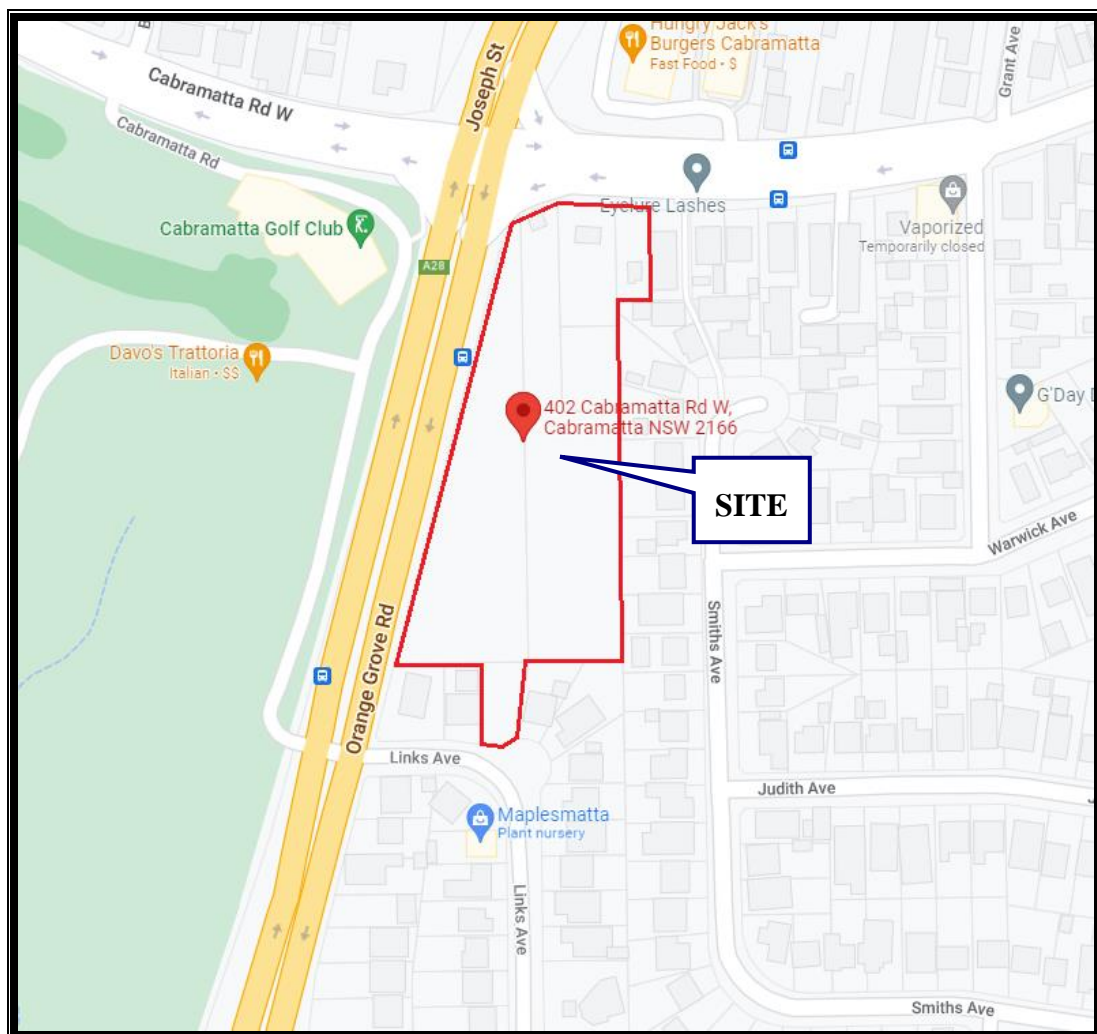


Figure 1: Site Locality Map

➤ ***Existing Site & Surrounding Land Use***

The subject site has a total area of 15,349m² and is currently occupied by vacant land. It is located in a mixed residential and commercial area, characterised by residential dwellings and is in close proximity to some retail and commercial sites. The site is located approximately 2 km from Cabramatta Railway Station and 3.2 km from Warwick Farm Railway Station.



Photo 1: Site frontage to Cabramatta Road West



Photo 2: Site frontage to Orange Grove Road



Photo 3: Site frontage to Links Avenue

3 EXISTING TRAFFIC CONDITIONS

3.1 Road Network and Classification

Cabramatta Road West is a state road that generally runs in an east to west direction, between Railway Parade (regional road) to the east and Elizabeth Drive (state road) to the west. Cabramatta Road West intersects with Orange Grove Road (state road) at the subject site.

3.2 Road Description and Traffic Control

Cabramatta Road West has a two-way divided carriageway with a width between kerbs of approximately 21 metres. This carriageway generally provides two travel lanes per direction. At present, parking is not permitted along Cabramatta Road West in the vicinity of the subject site, as per the signposted 'Clearway' between 6.00am and 7.00pm Mondays to Fridays and between 9.00am and 6.00pm on weekends and public holidays, as well as the signposted 'No Stopping' in the vicinity of the subject site.

The legal speed limit on Cabramatta Road West is signposted at 60km/h, with the exception of the signposted 'School Zone' of 40km/h during morning and afternoon school peak periods. Cabramatta Road West intersects with Orange Grove Road at the subject site and is controlled by traffic signals.



Figure 2: Aerial photograph of the subject site and surrounding road network



Photo 3: Cabramatta Road West at the subject site – facing east



Photo 4: Cabramatta Road West at the subject site – facing west

Orange Grove Road has a two-way divided carriageway, with a width between kerbs of approximately 24 metres. This carriageway generally provides three travel lanes per direction. At present, parking is not permitted on Orange Grove Road in the vicinity of the subject site, as per the signposted 'Clearway' between 6.00am and 7.00pm Mondays to Fridays and between 9.00am and 6.00pm on weekends and public holidays, as well as the signposted 'No Stopping' and 'No Parking' in front of the subject site. The legal speed limit on Orange Grove Road is signposted at 70km/h.



Photo 5: Orange Grove Road at the subject site – facing north



Photo 6: Orange Grove Road at the subject site – facing south

Links Avenue has a two-way undivided carriageway, with a width between kerbs of approximately 9 metres. This carriageway generally provides one travel lane per direction with kerbside parking available on both sides of the road. At present, unrestricted parking is permitted on Links Avenue, with the exception of the signposted 'No Stopping' at its near intersection with Orange Grove Road. The legal speed limit on Links Avenue is signposted at 50km/h.



Photo 7: Links Avenue at the subject site – facing south-east



Photo 8: Links Avenue at the subject site – facing west

3.3 Current Traffic Flows

A traffic volume count was undertaken by R.O.A.R. DATA Pty Ltd on behalf of Hemanote Consultants at the intersection of Orange Grove Road / Links Avenue / Access Road to Golf Club in the vicinity of the subject site on Tuesday 9 August 2022, during morning period (7.00am to 9.00am) and afternoon period (3.00pm to 6.00pm), considering traffic peak periods.

The current traffic flows in the morning & afternoon peak hours are shown in Table 1 below and Appendix 'C' of this report.

Traffic movement	Morning Peak Hour (Vehicles Per Hour)	Evening Peak Hour (Vehicles Per Hour)
	7.45am – 8.45am	3.45pm – 4.45pm
Orange Grove Road (North of Links Avenue) – 3 lanes		
Northbound	1,529	1,924
Southbound	1,715	1,851
Orange Grove Road (South of Links Avenue) – 3 lanes		
Northbound	1,512	1,922
Southbound	1,716	1,824
Links Avenue (East of Orange Grove Road)		
Eastbound	11	38
Westbound	32	16
Access Road to Golf Club (West of Orange Grove Road)		
Eastbound	4	2
Westbound	7	5

Table 1: Current Peak traffic flows in the vicinity of the subject site (on a typical weekday)

The results of the traffic volume counts undertaken determined that the traffic morning peak period was between 7.45am to 8.45am and the afternoon peak period was between 3.45pm to 4.45pm on a typical weekday.

The existing traffic flows on Orange Grove Road are typical for a state road in a mixed residential and commercial area.

The existing traffic flows on Links Avenue are low and typical for a local residential street, where traffic is controlled by the existing signalised intersection with Orange Grove Road.

It is determined that the existing mid-block level of service on Links Avenue is it level 'A', in accordance with Table 4.4 of the Roads & Maritime Services' *"Guide to Traffic Generating Developments - 2002"*, where peak hour flow is less than 200 vehicles/hr.

Level of Service	One Lane (veh/hr)	Two Lanes (veh/hr)
A	200	900
B	380	1400
C	600	1800
D	900	2200
E	1400	2800

Table 4.4: Urban road peak hour flows per direction RMS Guide)

➤ **Current Intersection Performance**

Average Vehicle Delay (AVD) and Level of Service (LOS) – The AVD and LOS provide a measure of the operational performance of an intersection, as indicated in Table 4.2 of the Roads & Maritime Services *"Guide to Traffic Generating Developments - 2002"* (shown below).

Level of Service	Average Delay per Vehicle (secs/veh)	Traffic Signals, Roundabout	Give Way & Stop Signs
A	< 14	Good operation	Good operation
B	15 to 28	Good with acceptable delays & spare capacity	Acceptable delays & spare capacity
C	29 to 42	Satisfactory	Satisfactory, but accident study required
D	43 to 56	Operating near capacity	Near capacity & accident study required
E	57 to 70	At capacity; at signals, incidents will cause excessive delays Roundabouts require other control mode	At capacity, requires other control mode

Table 4.2: Level of Service Criteria for intersections (RMS Guide)

A SIDRA intersection performance analysis was undertaken for the existing cross-intersection of Orange Grove Road / Links Avenue in the vicinity of the subject site (Pre-development).

Refer to Figure 3 below, showing the existing cross-intersection layout controlled by traffic signals.

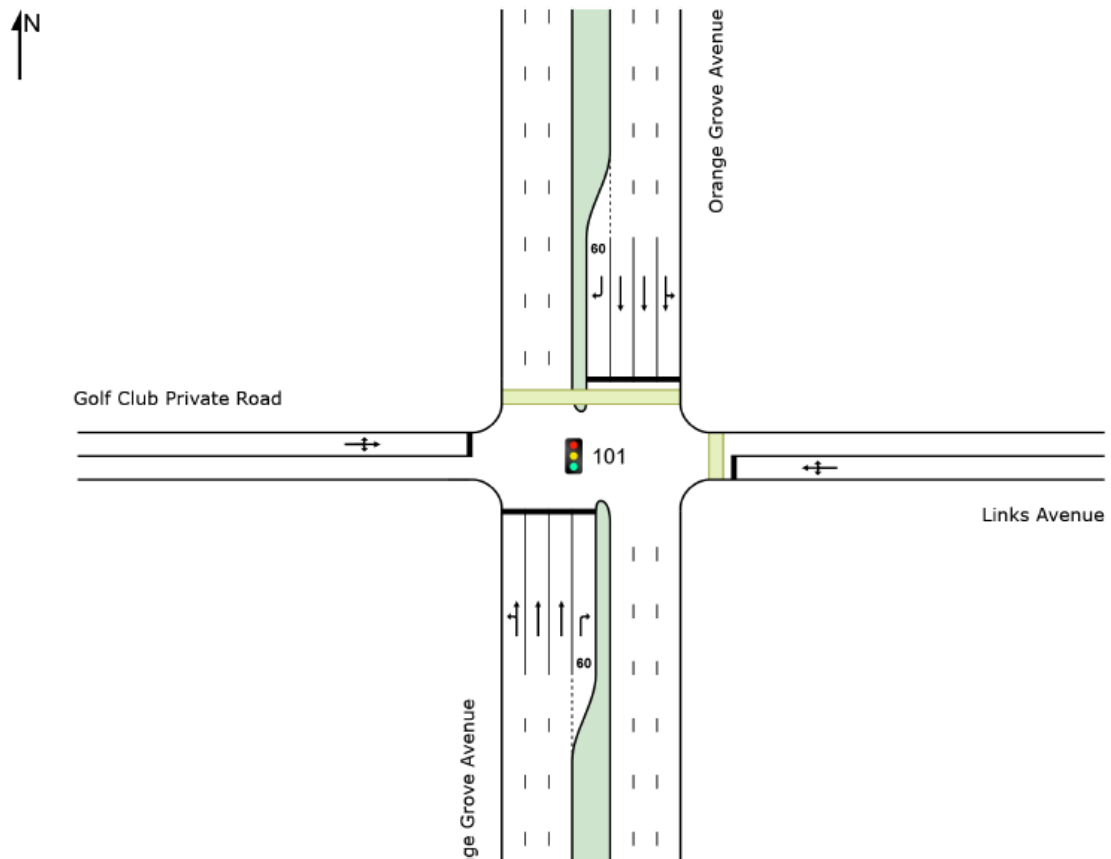


Figure 3: Existing intersection layout

The SIDRA performance analysis determined that the current operational performance during the AM peak period for the Orange Grove Road north approach leg is generally operating near capacity at a level of service 'D', with the exception of the right turn movement from Orange Grove Road into Links Avenue which is at level 'E'.

The existing operational performance during the AM peak period of the Orange Grove Road south approach is generally in good operation at a level of service 'A'. However, the left turn movement from Orange Grove Road into Golf Club Private

Road is at a level of service 'B' and the right turn movement from Orange Grove Road into Links Avenue is at level 'C'.

The existing operational performance during the AM peak period of the Links Avenue east approach leg is at a level of service 'D'.

The SIDRA performance analysis determined that the current operational performance during the PM peak period for the Orange Grove Road north approach leg is generally operating near capacity at a level of service 'E', with the exception of the right turn movement from Orange Grove Road into Links Avenue which is at level 'F'.

The current operational performance during the PM peak period of the Orange Grove Road south approach is generally in good operation at a level of service 'A'. However, the left turn movement from Orange Grove Road into Golf Club Private Road is at a level of service 'B' and the right turn movement from Orange Grove Road into Links Avenue is at level 'C'.

The current operational performance during the PM peak period of the Links Avenue east approach leg is at a level of service 'E'.

Refer to the summary of the results of the SIDRA intersection performance analysis attached in Appendix 'D' of this report.

3.4 Existing Transportation Services

The subject site has good access to existing public transport services in the form of trains and buses. The site is located approximately 2 km from Cabramatta Railway Station and approximately 3.2 km from Warwick Farm Railway Station.

Frequent bus services operate along Cabramatta Road West and Orange Grove Road, in close proximity to the subject site (i.e. bus routes 801, 815, 816 and 819).

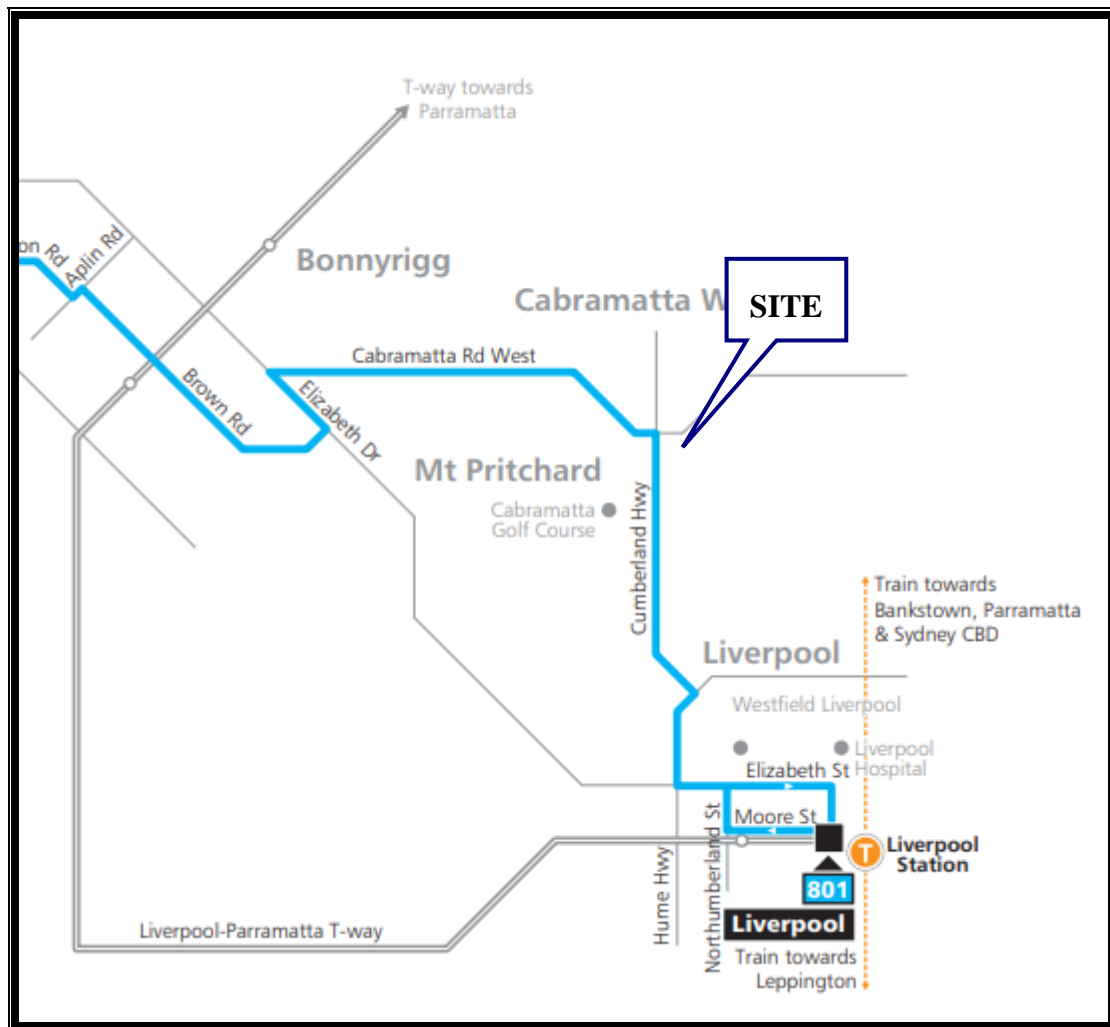
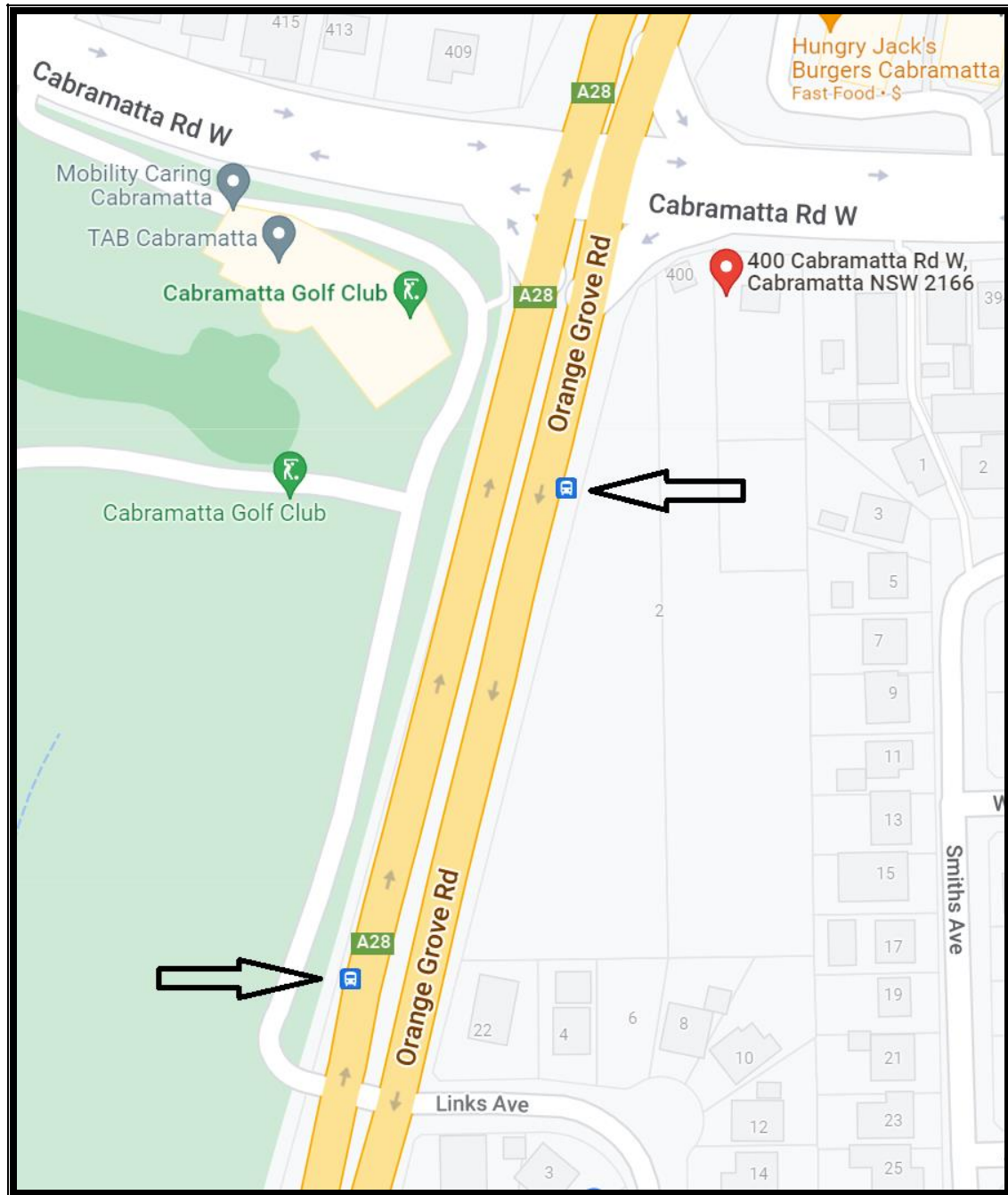


Figure 3: Bus services near the subject site (Bus no. 801)



Existing Bus stations near the subject site

4 PROPOSED DEVELOPMENT

4.1 *Description of the proposal*

The proposal is for the construction of a residential flat building and townhouses development, to be located at **400-404 Cabramatta Road West, 2-18 Orange Grove Road and 6 Links Avenue, Cabramatta**, with at-grade and basement parking levels.

The proposed development will include the following:

- A Residential Flat Building, with a total of eighty-seven (87) residential units consisting of:
 - 30 x 1-bedroom apartments.
 - 56 x two-bedroom apartments (including 9 adaptable units).
 - 1 x three-bedroom apartment.
- A total of one-hundred and nine (109) on-site car parking spaces, with 87 spaces for resident parking (including 9 adaptable parking spaces) and 22 spaces for visitor parking (including 3 accessible spaces with adjacent shared areas and a car wash bay), in addition to forty-four (44) bicycle storage spaces and a turning bay, over two basement parking levels and 9 visitor bicycle spaces on ground floor. A truck loading bay is located externally on ground level, accommodating up to a Heavy Rigid Vehicle (HRV – 12.5 metres in length).
- Fifty-three (53) residential townhouse dwellings consisting of:
 - 34 x three-bedroom townhouses (including 6 adaptable dwellings).
 - 19 x four-bedroom townhouses.
- A total of one-hundred and fifty-four (154) on-site car parking spaces allocated for the townhouses, with 59 open spaces and 48 enclosed garages (including 1 double garage and 6 adaptable garages) located on ground level, and 47 car spaces in basement level. **A total of 106 car spaces are allocated for residents (2 car spaces per townhouse) and 48 car spaces for visitors.**

Refer to **Appendix 'A'** for the proposed development plans.

4.2 Vehicular & Pedestrian Access

The vehicular access to and from the off-street parking facilities will be via a new access driveway crossing to be located in Links Avenue. The access driveway is to have a width of 7.7 metres, which is adequate for a low volume (Category 1) access driveway in accordance with AS2890.1:2004 and waste collection truck access.

The access driveway is to provide two-way vehicular movements, where two vehicles can pass each other at the same time without causing delays or congestion to traffic on the street. The proposed access driveway is located more than 6 metres from the tangent point of the adjacent kerblines, in accordance with Figure 3.1 of AS2890.1:2004.

Vehicular access is to be located and constructed in accordance with the requirements of AS2890.1:2004, where vehicles enter and exit the site in a forward direction at all times.

The existing vehicular crossings located in Orange Grove Road and Cabramatta Road West are to be removed and replaced with new kerb, gutter and footpath, to be constructed to Council specifications, to restore on-street car parking spaces.

The clear sight line triangle (2.5m x 2m) between the driver's eye view and pedestrians is to be provided on the exit side of the driveway, as per Figure 3.3 of AS2890.1:2004 and Figure 3.4 of AS2890.2:2018. Separate pedestrian access gates are also provided at the front of the site in Cabramatta Road West and Orange Grove Road, to segregate pedestrians and vehicles and improve safety within the site.

4.3 On-site Parking Provision

Fairfield CityWide Development Control Plan 2013, Chapter 12, Section 12.1, Table 1, requires off-street parking for multi-dwelling housing located less than 400m from a major bus station to be provided at a rate of:

- 1-2 bedrooms (less than 110m²) – 1 space.
- 3 or more bedrooms (greater than 110m²) – 1.5 spaces.
- 0.25 visitor spaces per dwelling.

Refer to Table 2 below for the required and proposed car parking provision for the proposed **townhouses**:

Proposed breakdown	Car parking rate	Proposed No. of units	Car parking required	Total car parking required	Total car parking provided
Multi-Dwelling Housing					
1-2 bed apartments (less than 110m ²)	1 space	-	-	94	154
3 or more bed apartments (greater than 110m ²)	1.5 spaces	53	80		
Visitor	0.25 spaces per dwelling	53	14		
				94	154
Compliance with on-site car parking					Yes

Table 2: On-site parking requirement and provision (Townhouses)

The proposed development provides for fifty-three (53) residential dwellings (including 34 x 3-bedroom dwellings and 19 x 4-bedroom dwellings), and therefore requires a minimum of 94 car parking spaces, as outlined above, including a minimum of 14 visitor car spaces and 80 resident car spaces.

The proposed development provides one-hundred and fifty-four (154) on-site car parking spaces allocated for the townhouses, with 59 open spaces and 48 enclosed garages (including 1 double garage and 6 adaptable garages) located on ground

level, and 47 car spaces in basement level. **A total of 106 car spaces are allocated for residents (2 car spaces per townhouse) and 48 car spaces for visitors.**

Fairfield CityWide Development Control Plan 2013, Chapter 12, Section 12.1, Table 1, requires off-street parking for residential flat buildings to be provided at a rate of:

- 1 space per dwelling, plus
- 1 visitor space per 4 dwellings where a development has more than 2 proposed dwellings.

Refer to Table 3 below for the required and proposed car parking provision for the proposed **RFB**:

Proposed breakdown	Car parking rate	Proposed No. of units	Car parking required	Total car parking required	Total car parking provided
Residential Flat Building					
Dwellings	1 space per dwelling	87	87	109	109
Visitor	1 space per 4 dwellings		22		
Total				109	109
Compliance with on-site car parking					Yes

Table 3: On-site parking requirement and provision (RFB)

The proposed development provides for eighty-seven (87) residential units (including 30 x 1-bedroom units, 56 x 2-bedroom units and 1 x 3-bedroom units), and therefore requires a total of 109 car parking spaces.

The proposed development provides one-hundred and nine (109) on-site car parking spaces, with 87 spaces for resident parking (including 9 adaptable parking spaces) and 22 spaces for visitor parking (including 3 accessible spaces with adjacent shared areas and a car wash bay), in addition to forty-four (44) bicycle storage spaces and a turning bay, over two basement parking levels and 9 visitor bicycle spaces on ground floor. A truck loading bay is located externally on ground level, accommodating up to a Heavy Rigid Vehicle (HRV – 12.5 metres in length). **Therefore, the proposed on-site parking provision is adequate for the proposed development and in compliance with Council's parking requirements.**

4.4 On-site Parking Layout and Circulation

The layout of the on-site car parking area and manoeuvring arrangements has been designed to enhance vehicular and pedestrian access, where vehicles enter and exit the site in a forward direction, through the provision of adequate internal aisle width and turning space.

AS2890.1:2004 Parking facilities Part 1: Off-street car parking requires a minimum parking space width of 2.4 meters (for User Class 1A – long-term residential parking) and 2.5 metres (for User Class 2 - medium-term visitor parking) and a minimum length of 5.4 meters. The proposed on-site car parking spaces have a minimum width of 2.4 metres (for residential parking) and a width of 2.5 metres (for visitor parking) and a minimum length of 5.4 meters each, which is adequate.

The accessible car parking spaces have a width of 2.4 metres, in addition to an adjacent 2.4 metres wide shared area, which is adequate in accordance with AS2890.6:2009.

The adaptable car parking spaces (for the RFB) have a width of 3.8 metres, which is adequate.

The single enclosed garages (for the townhouses) have an internal width of 3.1 metres, with the exception of 6 adaptable garages which have an internal width of 3.8 metres and the double garage at grade level, which has a width of 6 metres. The double enclosed garages located within the basement have an internal width of 5.8 metres, which is adequate

The loading bay located on ground level in a central position on the site, has a clear width of 3.5 metres and a length of 12.5 metres, which is adequate for HRV access in accordance with AS2890.2:2018.

Car parking spaces adjacent to walls or obstructions have been made wider than the minimum width, to accommodate full door opening in accordance with Clause 2.4.2(d) of AS2890.1:2004.

Clause 2.4.2 of AS2890.1:2004 requires a minimum aisle width of 5.8 metres for two-way aisles, adjacent to 90° angle parking. The proposed aisles have a minimum

width of 5.8 metres, which is adequate for two-way traffic and manoeuvring into and out of parking spaces.

A turning area is also provided towards the rear of the basement carpark level for the townhouses and the basement 2 carpark level for the RFB, to allow vehicles to turn around and exit in a forward direction, if all other car parking spaces are occupied.

The ramp to the basement level of the RFB has a clear width of 6.5 metres at the top of the ramp which narrows down to a width of 5.5 metres, in addition to a 300mm kerb on either side and has a maximum grade of 1:20 (5%) for the first 6 metres within the site. It has a maximum grade of 1:4 (25%) with a change of grade of 1:8 (12.5%) over the 2 metres at either end of the ramp, which is adequate.

The ramp to the basement level for the townhouses is adequate and complies with AS2890.1:2004

A minimum 2.2 metres headroom clearance is to be provided from the car park basement levels to the underside of all services conduits and suspended stormwater pipelines, in accordance with Clause 5.3.1 of AS2890.1:2004. A "Maximum Headroom Clearance 2.2m" sign is to be erected at the entrance to the basement car park areas and is to be clearly visible to all drivers. A minimum 2.5 meters headroom clearance is to be provided above the accessible parking spaces and adjacent shared zones as well as the adaptable car parking spaces in accordance with Clause 2.4 of AS2890.6:2009.

Traffic convex mirrors are to be provided at the bottom of the vehicular ramps, to provide drivers with further assistance with viewing oncoming traffic, as an additional safety and traffic management measure.

All vehicular manoeuvring within the site has been designed and checked using the HRV, B99 and B85 standard design vehicle turning paths from AS2890.1:2004, AS2890.2:2018 and Austroads. Refer to the vehicle swept paths diagrams attached in Appendix 'B' of this report.

Therefore, the car parking layout and vehicular circulation are adequate in accordance with AS2890.1:2004, AS2890.6:2009 and AS2890.2:2018, where vehicles are to enter and exit the site in a forward direction at all times.

4.5 Waste Collection & Deliveries

Waste collection will be undertaken on-site within the dedicated truck loading bay located in a central position in the car parking area on ground level, by the waste collection vehicle.

The truck loading bay will also be used to service the site for ongoing deliveries and maintenance.

5 EXTERNAL PARKING IMPACT

5.1 Existing Parking Controls

The subject site is located in a mixed residential and commercial area, where parking is not permitted along Cabramatta Road West in the vicinity of the subject site, as per the signposted 'Clearway' between 6.00am and 7.00pm Mondays to Fridays and between 9.00am and 6.00pm on weekends and public holidays, as well as the signposted 'No Stopping' in the vicinity of the subject site

In addition, parking is not permitted on Orange Grove Road in the vicinity of the subject site, as per the signposted 'Clearway' between 6.00am and 7.00pm Mondays to Fridays and between 9.00am and 6.00pm on weekends and public holidays, as well as the signposted 'No Stopping' and 'No Parking' in front of the subject site.

Further, unrestricted parking is permitted on Links Avenue, with the exception of the signposted 'No Stopping' at its near intersection with Orange Grove Road.

5.2 Impacts of Proposed Development on Parking

The parking demand resulting from the proposed residential development can be accommodated within the proposed adequate on-site car and bicycle parking spaces for residents and visitors. The subject site has great access to existing public transport in the form of trains and bus services.

Therefore, the proposed development will not have adverse impacts on parking in the surrounding area.

6 EXTERNAL TRAFFIC IMPACT

6.1 *Estimated Future Traffic Generation*

An indication of the potential traffic generation of the proposed development is provided by the RMS *Guide to Traffic Generating Development - 2002*.

The Guide specifies the following traffic generation rates for **medium density** residential developments:

Larger units and town houses (three or more bedrooms):

- Weekday peak hour vehicle trips = 0.5-0.65 per dwelling.

Therefore, the proposed townhouses with fifty-three (53) residential apartments (34 x 3-bed units and 19 x 4-bed units) has an estimated traffic generation as follows:

- 27 to 35 peak hour vehicle trips (In and Out trips).

The Guide also specifies the following traffic generation rates for **high density** residential developments:

- AM peak (1 hour) vehicle trips per unit = 0.19.
- PM peak (1 hour) vehicle trips per unit = 0.15.

Therefore, the proposed RFB with eighty-seven (87) residential units (30 x 1-bed units, 56 x 2-bed units and 1 x 3-bed unit) has an estimated traffic generation as follows:

- 17 AM peak hour vehicle trips (In and Out trips).
- 13 PM peak hour vehicle trips (In and Out trips).

The total estimated peak hour traffic generation is:

- 52 AM peak hour vehicle trips (In and Out trips).
- 48 PM peak hour vehicle trips (In and Out trips).

6.2 Projected Intersection Performance

Average Vehicle Delay (AVD) and Level of Service (LOS) – The AVD and LOS provides a measure of the operational performance of an intersection, as indicated in Table 4.2 of the Roads & Maritime Services “*Guide to Traffic Generating Developments - 2002*” (shown below).

Level of Service	Average Delay per Vehicle (secs/veh)	Traffic Signals, Roundabout	Give Way & Stop Signs
A	< 14	Good operation	Good operation
B	15 to 28	Good with acceptable delays & spare capacity	Acceptable delays & spare capacity
C	29 to 42	Satisfactory	Satisfactory, but accident study required
D	43 to 56	Operating near capacity	Near capacity & accident study required
E	57 to 70	At capacity; at signals, incidents will cause excessive delays Roundabouts require other control mode	At capacity, requires other control mode

Table 4.2: Level of Service Criteria for intersections (RMS Guide)

A post-development SIDRA intersection performance modelling analysis was undertaken for the cross-intersection of Orange Grove Road / Links Avenue in the vicinity of the subject site, and it was modelled as the proposed network layout as shown in Figure 5 on the following page.

Refer to the summary of the results of the SIDRA intersection performance analysis (undertaken for pre & post development, including the 10-year future growth) attached in Appendix ‘D’ of this report.

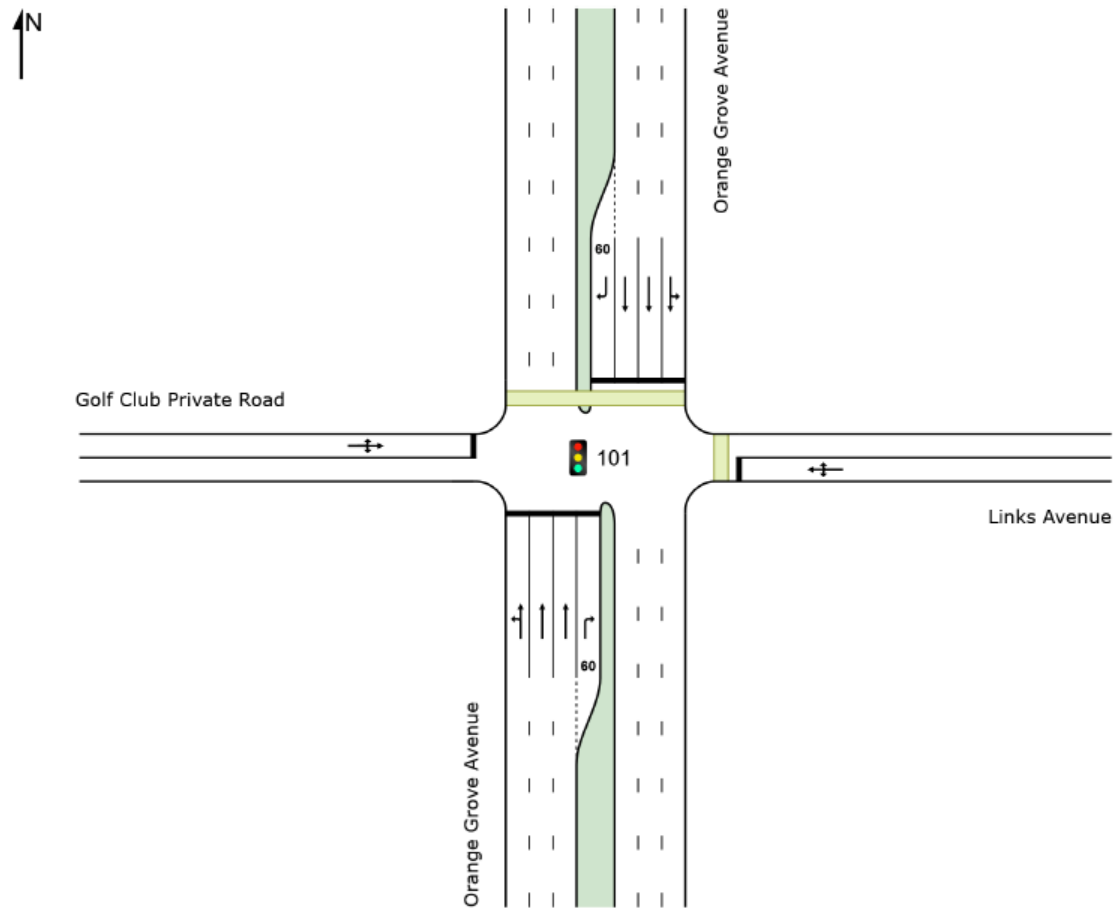


Figure 5: Intersection Layout

The following assumptions have been considered and adopted in the SIDRA Network Intersection modelling for the post-development conditions considering that the main access to and from the subject site is through Links Avenue:

- Morning and afternoon peak traffic generation from *RMS Guide to Traffic Generating Developments* has been used, as outlined in Section 6.1 of this report.
- AM & PM Traffic generated by the development was equally adopted for inbound and outbound traffic.
- Pre-development network analysis is modelled for the base year (2022) and 10 years of future growth (2032) in surrounding traffic. The annual traffic growth rate for the 10-year future period was based on the SIDRA intersection analysis software, which allows for future analysis in surrounding traffic by applying a uniform growth rate of 2% for each year over the 10-year period.
- Post-development network analysis is modelled for when the residential development is in operation and after 10 years of future growth in surrounding traffic.
- The entered traffic flow was inputted as 95% of the modelled flow.

These assumptions will result in the development trip distribution shown in Figures 6 and 7 for relevant traffic movement and modelled intersection.

TRIP DISTRIBUTION FROM
PROPOSED DEVELOPMENT
AM PEAK: 7.45AM - 8.45AM

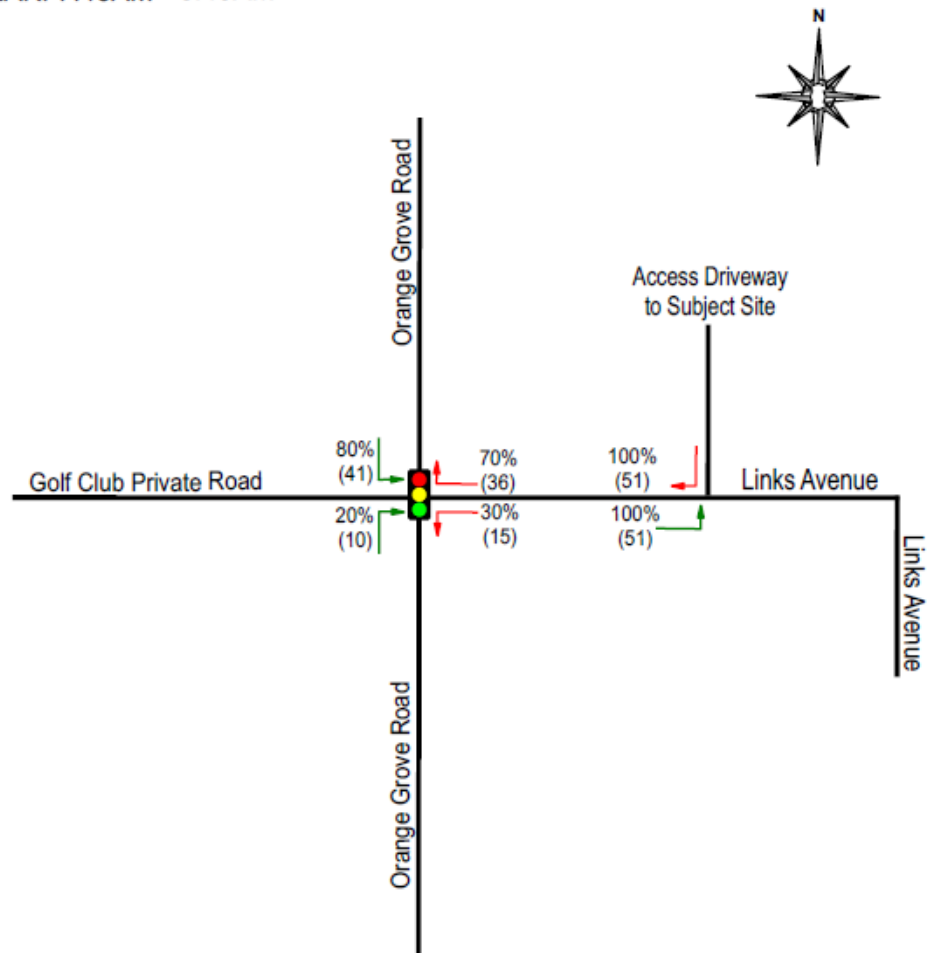


Figure 6: Development Traffic Distribution on the Surrounding Road Network – **AM Peak**

TRIP DISTRIBUTION FROM
PROPOSED DEVELOPMENT
PM PEAK: 3.45PM - 4.45PM



Figure 7: Development Traffic Distribution on the Surrounding Road Network – **PM Peak**

The outcome of the SIDRA modelling

A summary of the results of the SIDRA intersection performance analysis has been provided in Tables 4 and 5 below, as well as the SIDRA Movement Summary Tables attached in Appendix 'D' of this report.

Modelled Peak	Average LOS	Average Delay (sec)	DOS (Veh/C)
Base Year 2022 – Pre-Development	C	30.0	0.880
Base Year 2022 – Post Development	C	32.8	0.901
Future Year 2032 – Pre-Development	D	47.1	0.968
Future Year 2032 – Post Development	D	49.4	0.975

Table 4: Network SIDRA Modelling – Orange Grove Road / Links Avenue – 7.45am – 8.45am

Modelled Peak	Average LOS	Average Delay (sec)	DOS (Veh/C)
Base Year 2022 – Pre-Development	C	33.2	0.900
Base Year 2022 – Post Development	C	36.3	0.919
Future Year 2032 – Pre-Development	F	87.7	1.099
Future Year 2032 – Post Development	F	96.8	1.122

Table 5: Network SIDRA Modelling – Orange Grove Road / Links Avenue – 3.45pm – 4.45pm

It can be concluded from the results of the SIDRA modelling that:

- The current operational performance (pre-development) at the cross-intersection of Orange Grove Road / Links Avenue is currently operating at a Level of Service (LOS) ranging between 'A' and 'F' during weekday AM & PM peak.
- The proposed residential development will generally not alter the current LOS at the subject intersection, with the exception of the left turn movement into Links Avenue during the AM peak period which will change from level 'D' to 'E' (a minor increase of 4.1 seconds).
- The vehicle queue length on Links Avenue at the existing intersection will increase from 12.2m to 32.3m (an increase of 3 vehicles) and from 7.9m to 31.5m (an increase of 3 vehicles), during the AM and PM peak periods, respectively. This is acceptable and the post development queue length will be well within the distance between the proposed access driveway and the intersection, and will not impact on vehicular access for the subject site.
- The future 10-year (pre-development) analysis indicates that the subject intersection will operate at a LOS ranging between 'A' and 'F', during weekday AM & PM peak.
- The proposed residential development will generally not alter the future LOS at the subject intersection, with the exception of the right turn movement into Links Avenue during the AM peak period which will change from level 'C' to 'D' (a minor increase in 1.4 seconds).

Therefore, the estimated traffic generation from the proposed development is of low impact on existing flows on Orange Grove Road, Links Avenue and surrounding streets and will not have adverse impacts on the current operational performance of the subject existing intersection, which will generally continue to operate at the same level of service.

The additional traffic generated by the proposed development can be readily accommodated within the existing road layout, without adverse impacts on the amenity of the area.

7 RECOMMENDATIONS

The following measures are recommended for the proposed development, to increase traffic and pedestrian safety and reduce the reliance to travel by private cars and vehicle trips, in line with Clause 2.122 *Traffic Generating Development* of the State Environmental Planning Policy (Transport and Infrastructure) 2021.

- A Green Travel Plan be prepared for the subject site, to encourage residents to utilise existing public transport services in the local area and other modes of travel, in order to reduce the reliance on private vehicle trips, where possible.
- Install advisory warning signage at the entrance and throughout the subject site, to advise motorists of a signposted speed limit of 10km/h, 'Slow Down' and 'Watch for pedestrians' signage, to increase traffic and pedestrian safety.
- Install speed cushions where needed, to further reduce the speed environment within the site.
- Install adequate lighting within the subject site, to increase traffic and pedestrian safety.

8 CONCLUSION

It can be concluded from the traffic and parking impact assessment that the proposed residential flat building and townhouses development at 400-404 Cabramatta Road West, 2-18 Orange Grove Road and 6 Links Avenue, Cabramatta, will not have adverse impacts on existing traffic or parking conditions and is worthy of Council's support in its present form.

- The existing traffic flows on Orange Grove Road and Links Avenue are typical for a state road and a local road, respectively, located in a mixed residential and commercial area, where traffic is well controlled by the existing signalised intersection, with spare capacity.
- The estimated traffic generation from the proposed development is of low impact on existing flows on Orange Grove Road, Links Avenue and surrounding streets and will not have adverse impacts on the current operational performance of the subject existing intersection, which will continue to operate at similar and acceptable levels of service. The traffic generated by the proposed residential development can be readily accommodated within the existing road network.
- The potential increase in the number of vehicle movements in and about Orange Grove Road, Links Avenue and adjacent streets will not have adverse impacts on the amenity of the area.
- The parking demand resulting from the proposed residential development can be easily accommodated within the proposed adequate off-street car and bicycle parking for both residents and visitors, which is in compliance with AS2890.1:2004 and Council's parking requirements.
- The on-site vehicular access, car parking layout and vehicular circulation is adequate for the proposed development and in accordance with AS2890.1:2004, AS2890.6:2009 and AS2890.2:2018, where vehicles enter and exit the site in a forward direction at all times.
- The subject site has good access to existing public transport services.
- The proposed development will not have adverse impacts on parking in the surrounding area.

Appendix A – Proposed Development Plans (Residential Flat Building)







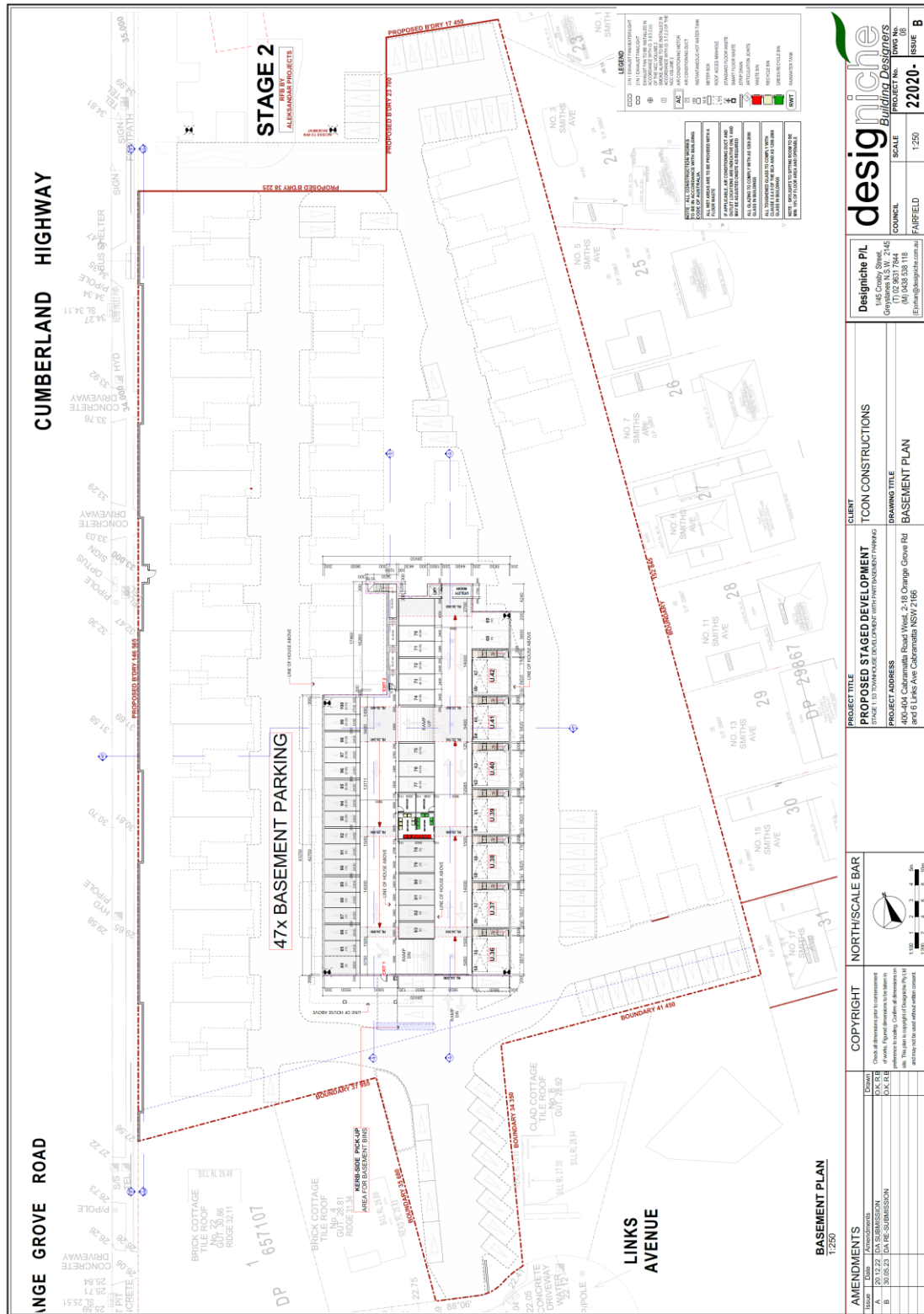




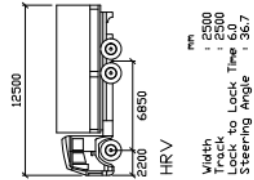


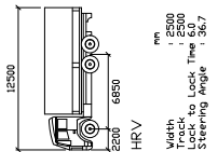
Appendix A – Proposed Development Plans (Townhouses)

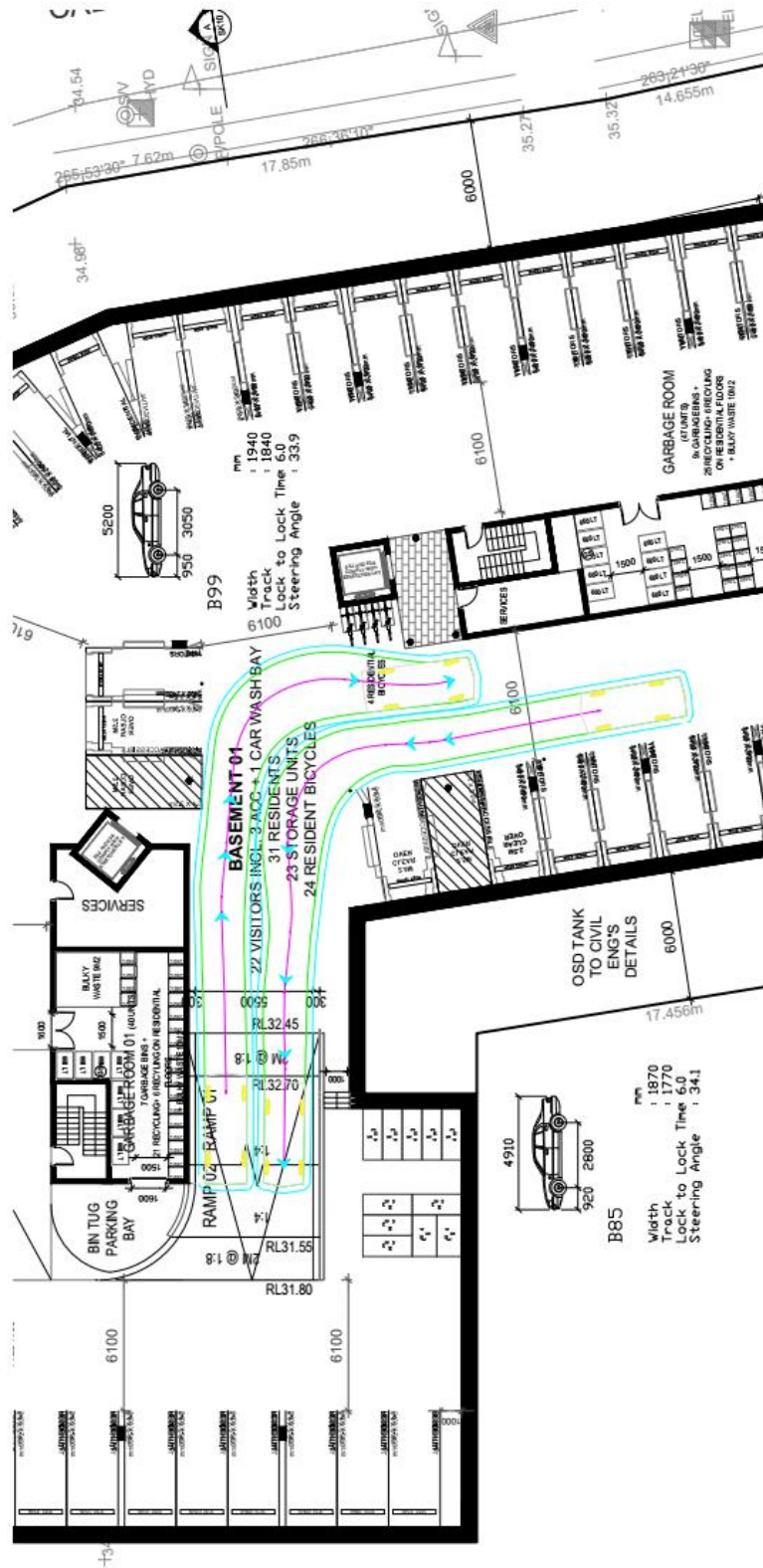


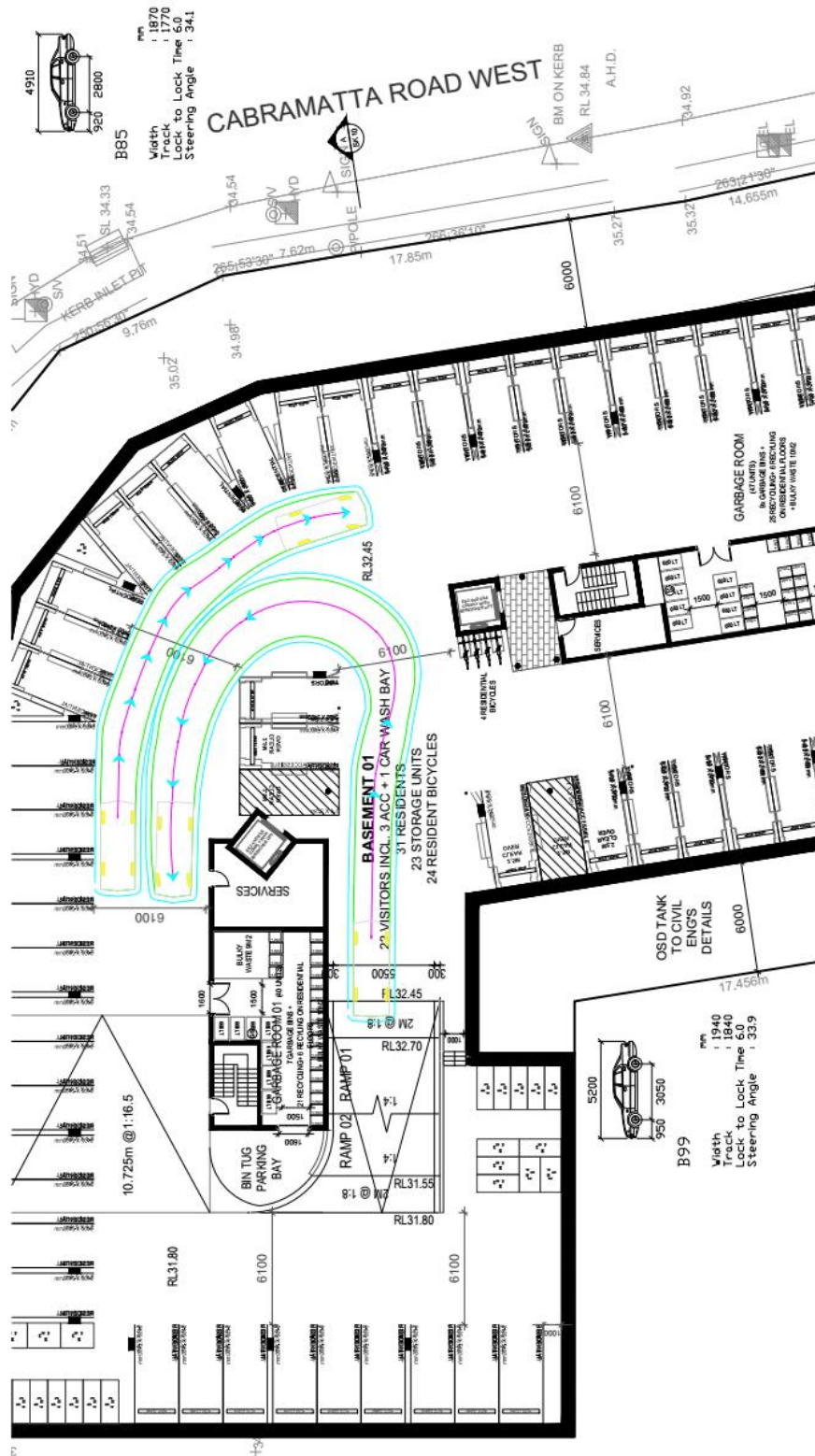


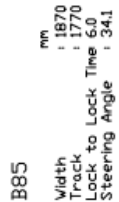
Appendix B – Vehicle Swept Paths (Residential Flat Building)

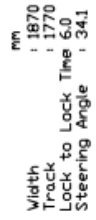


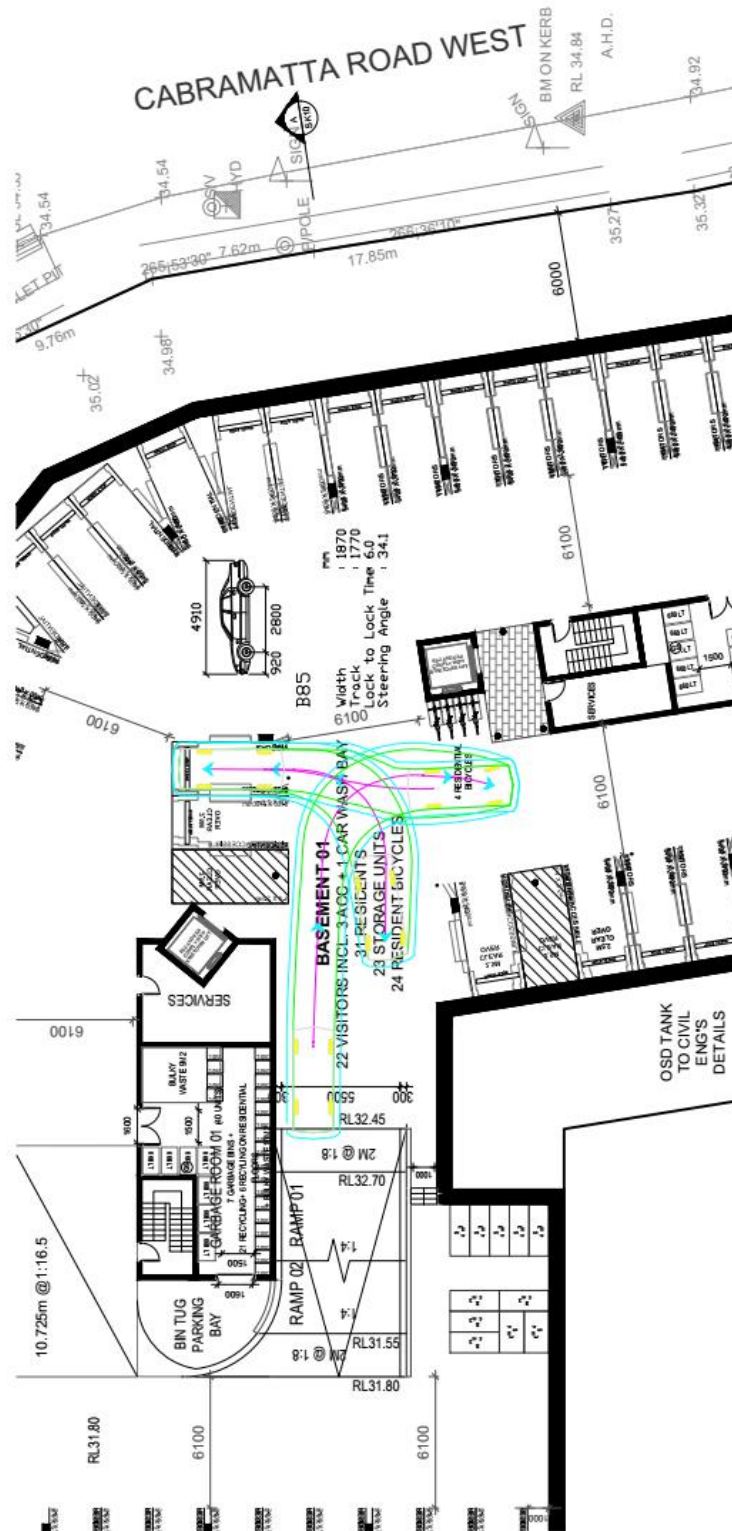


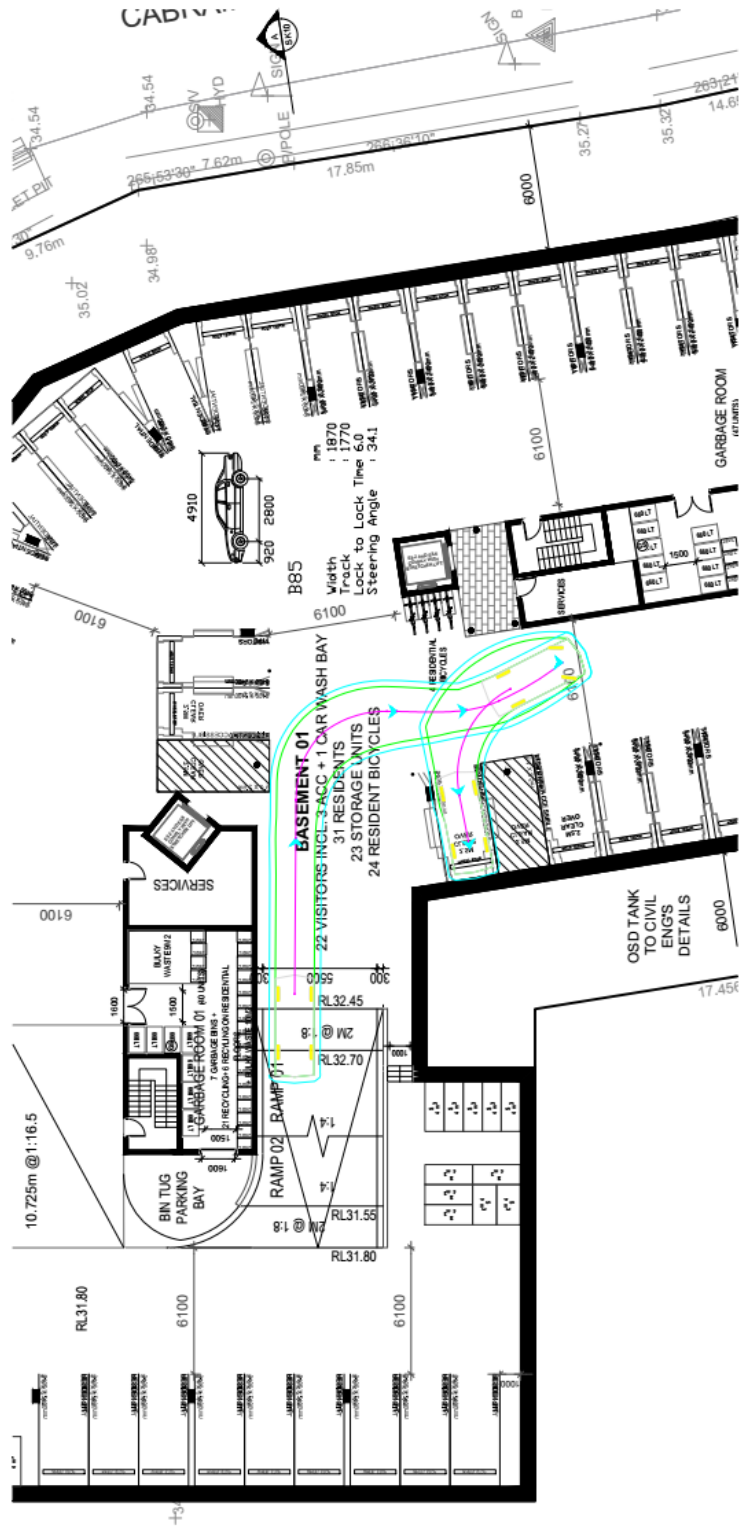


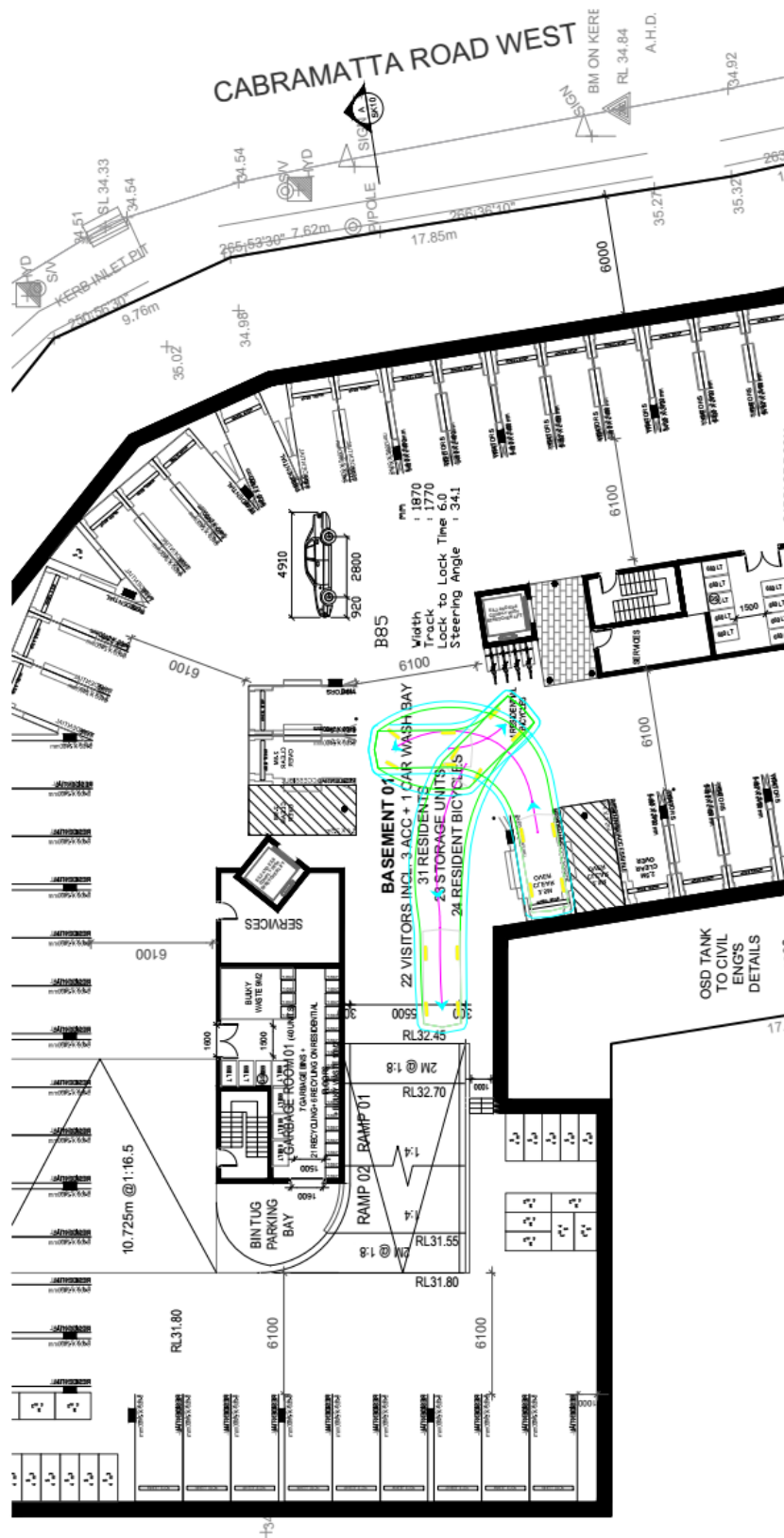


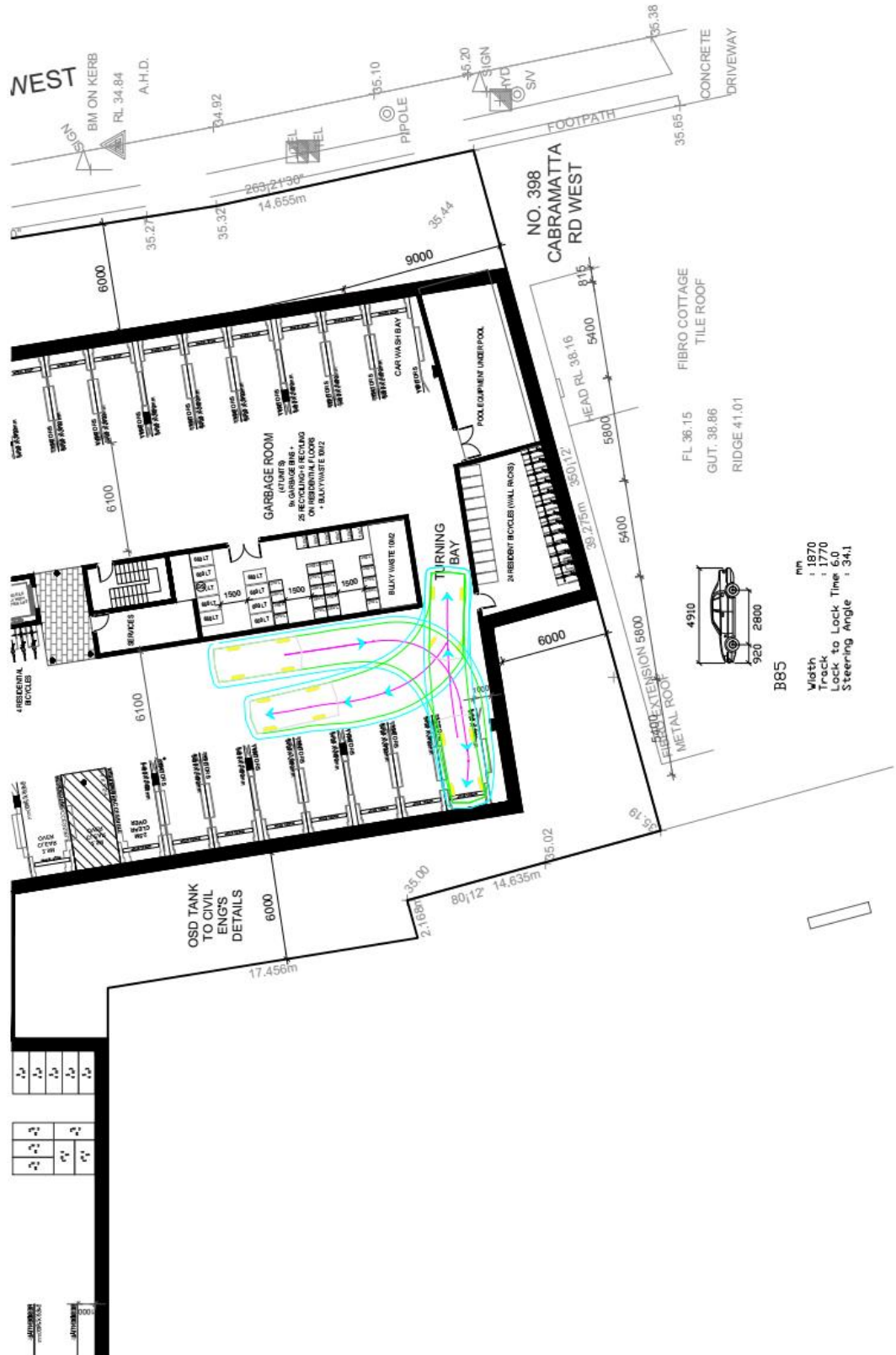


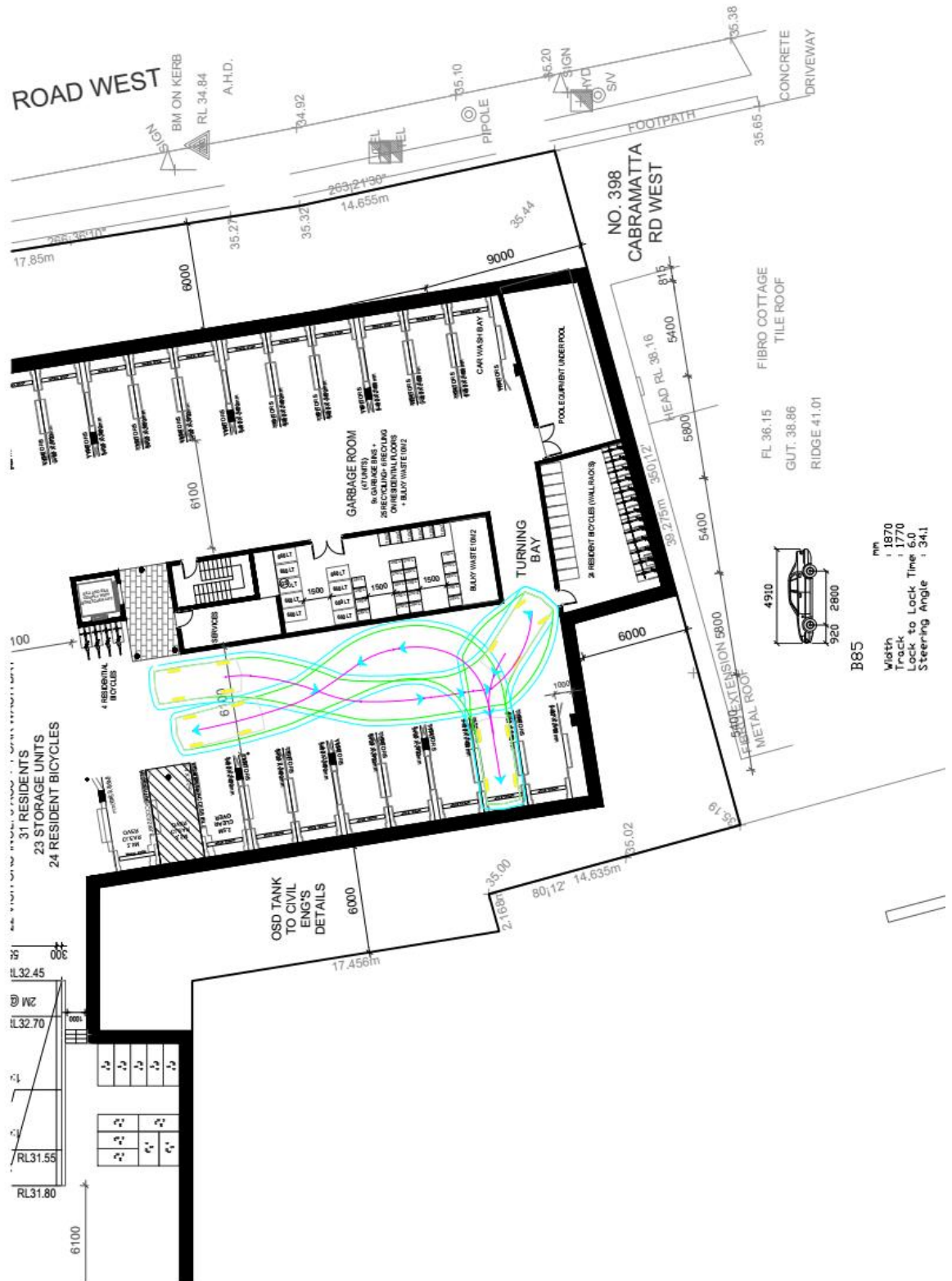


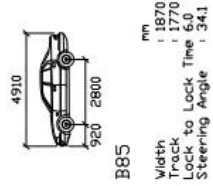


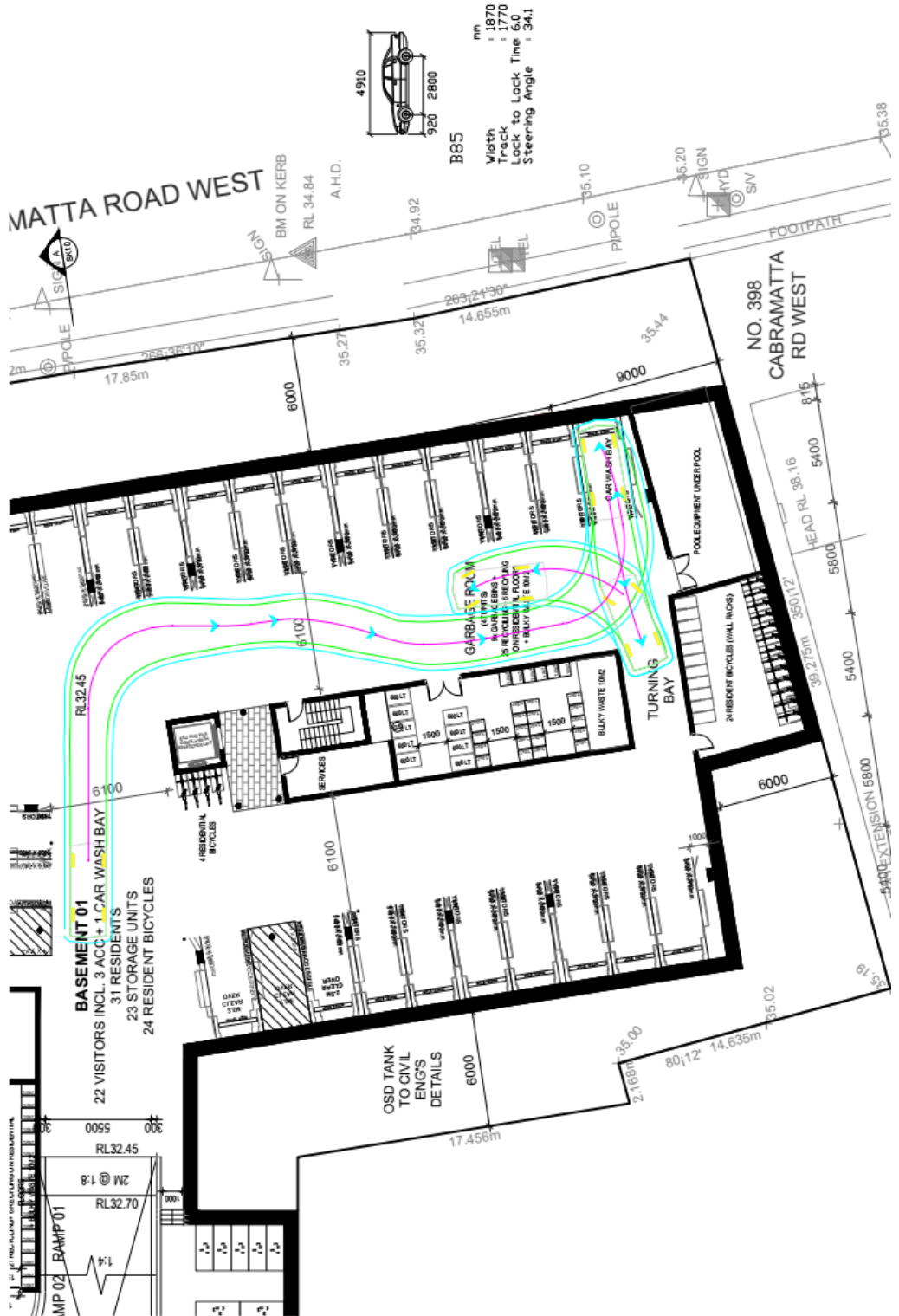


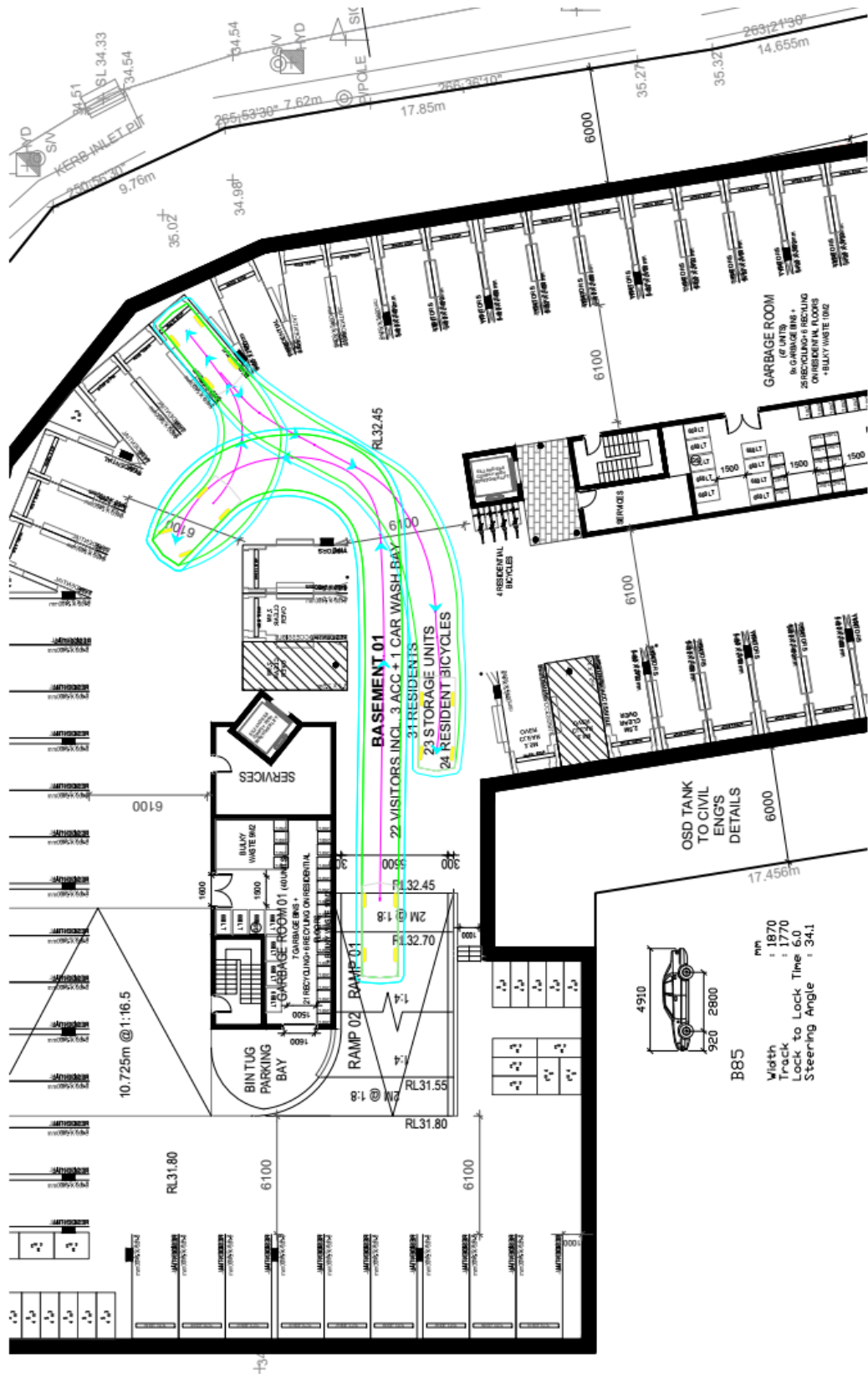


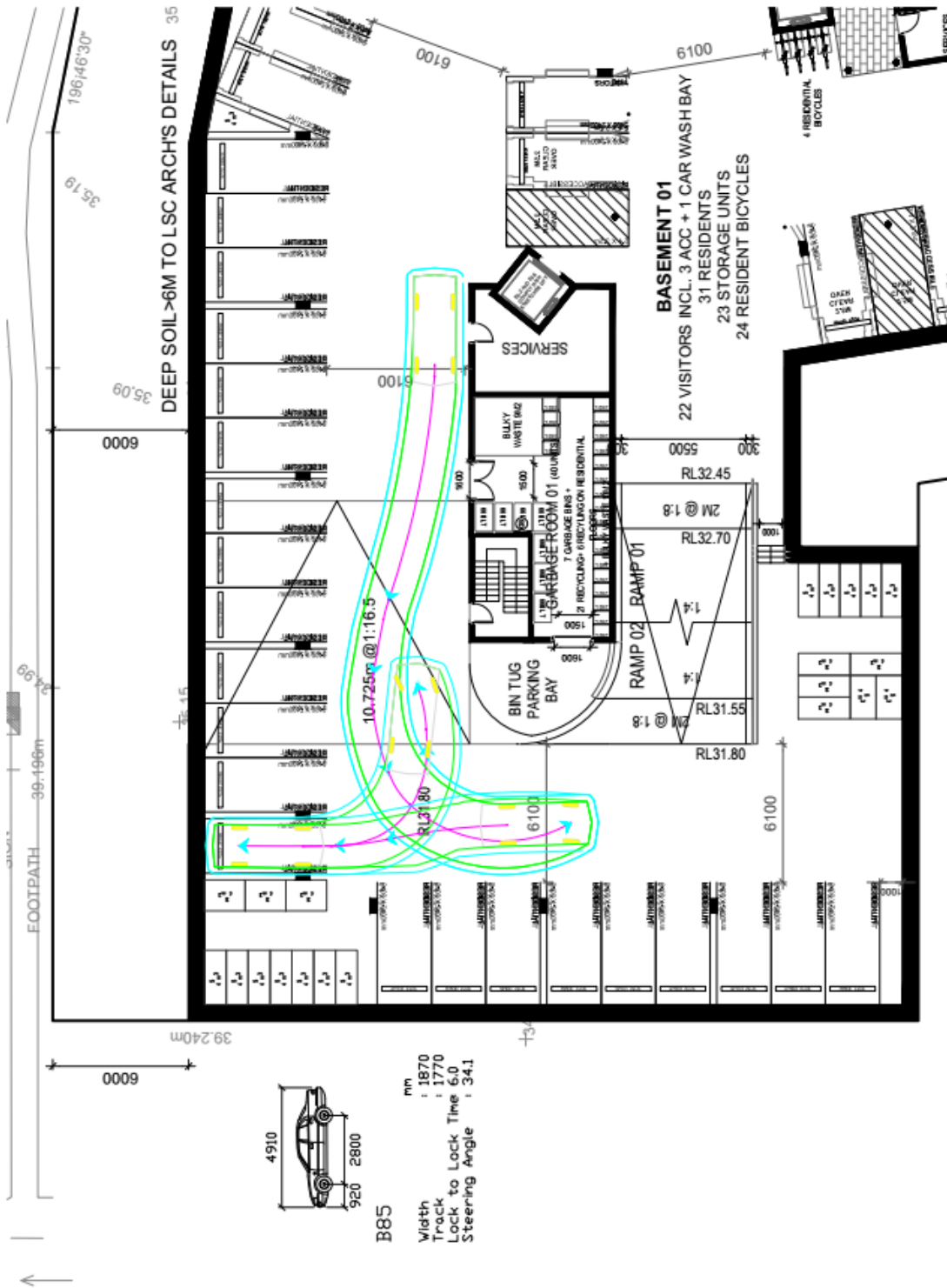


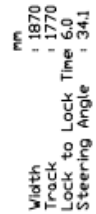


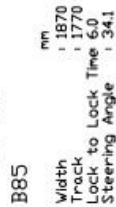




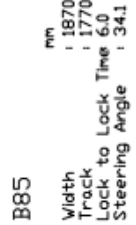


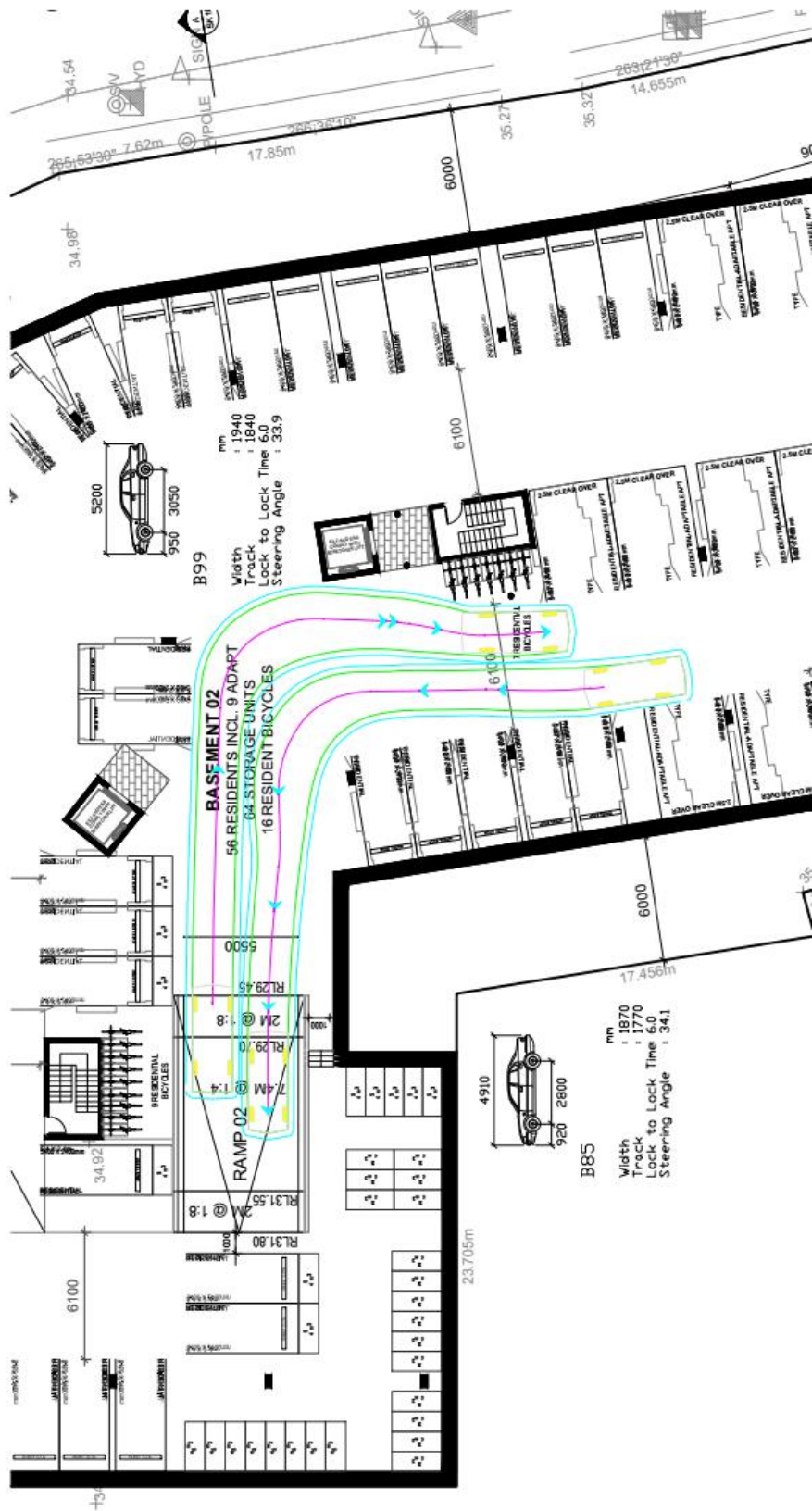


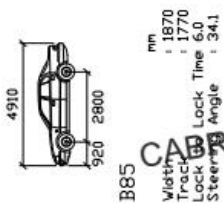


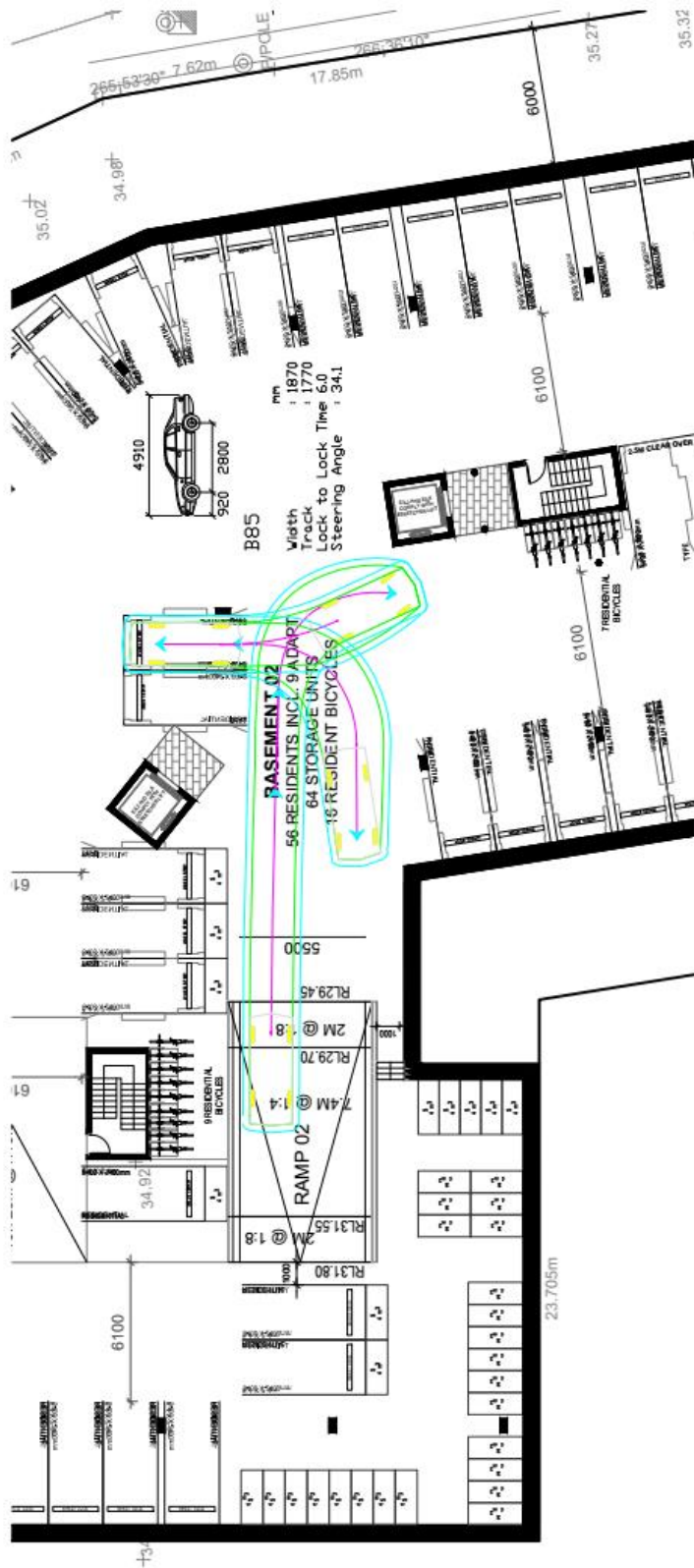


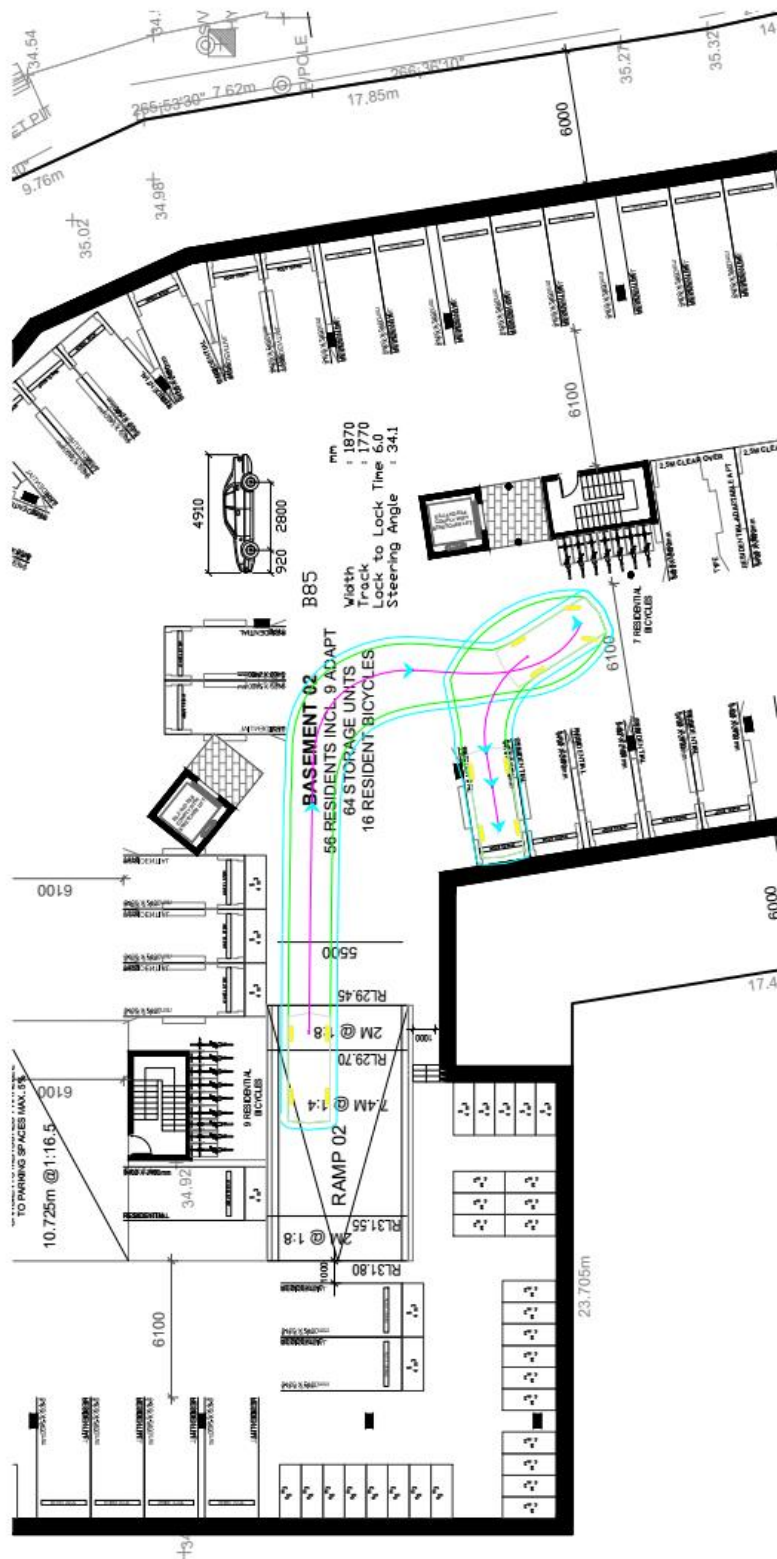


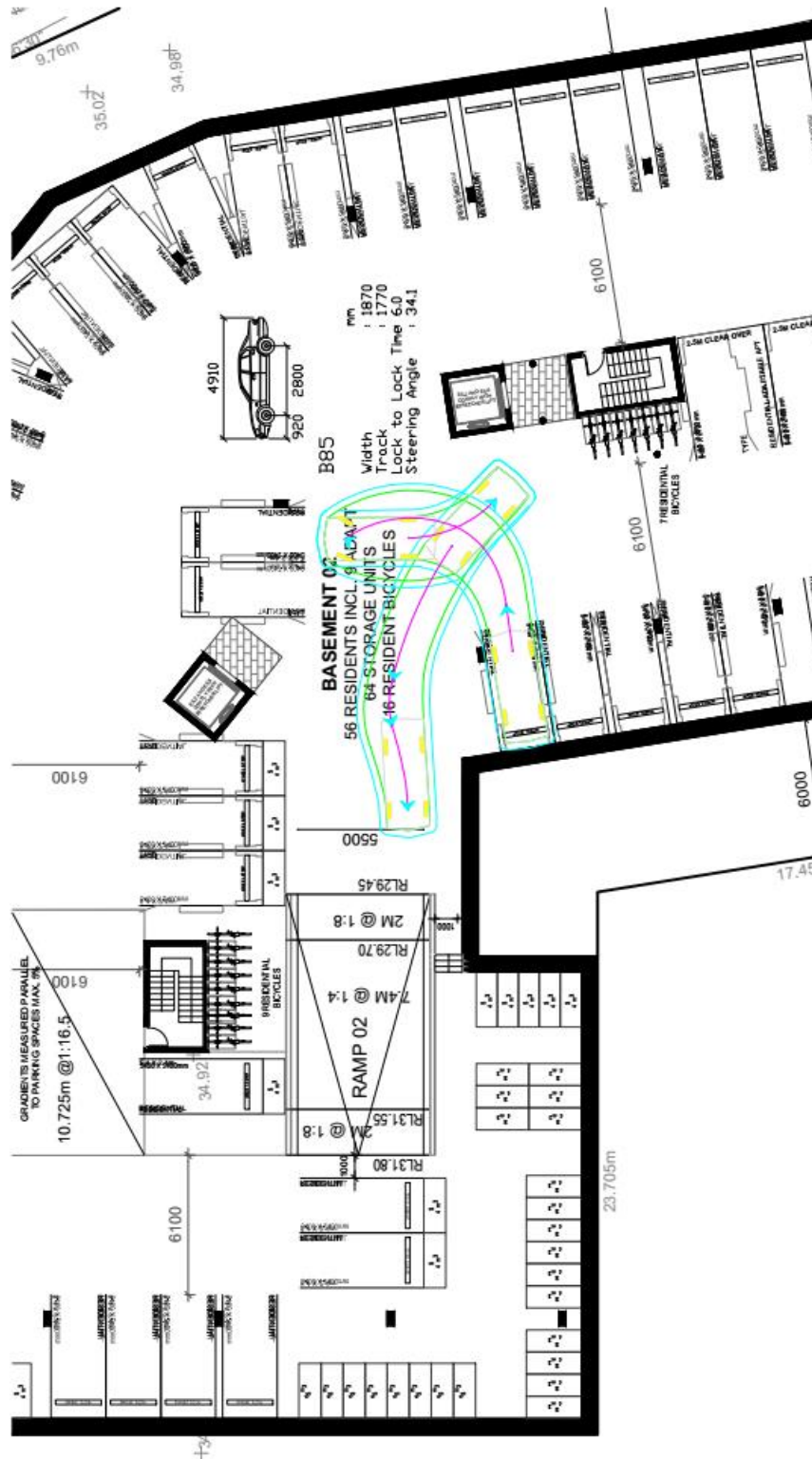


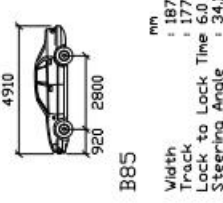




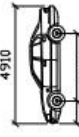


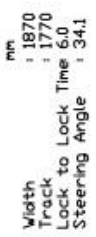


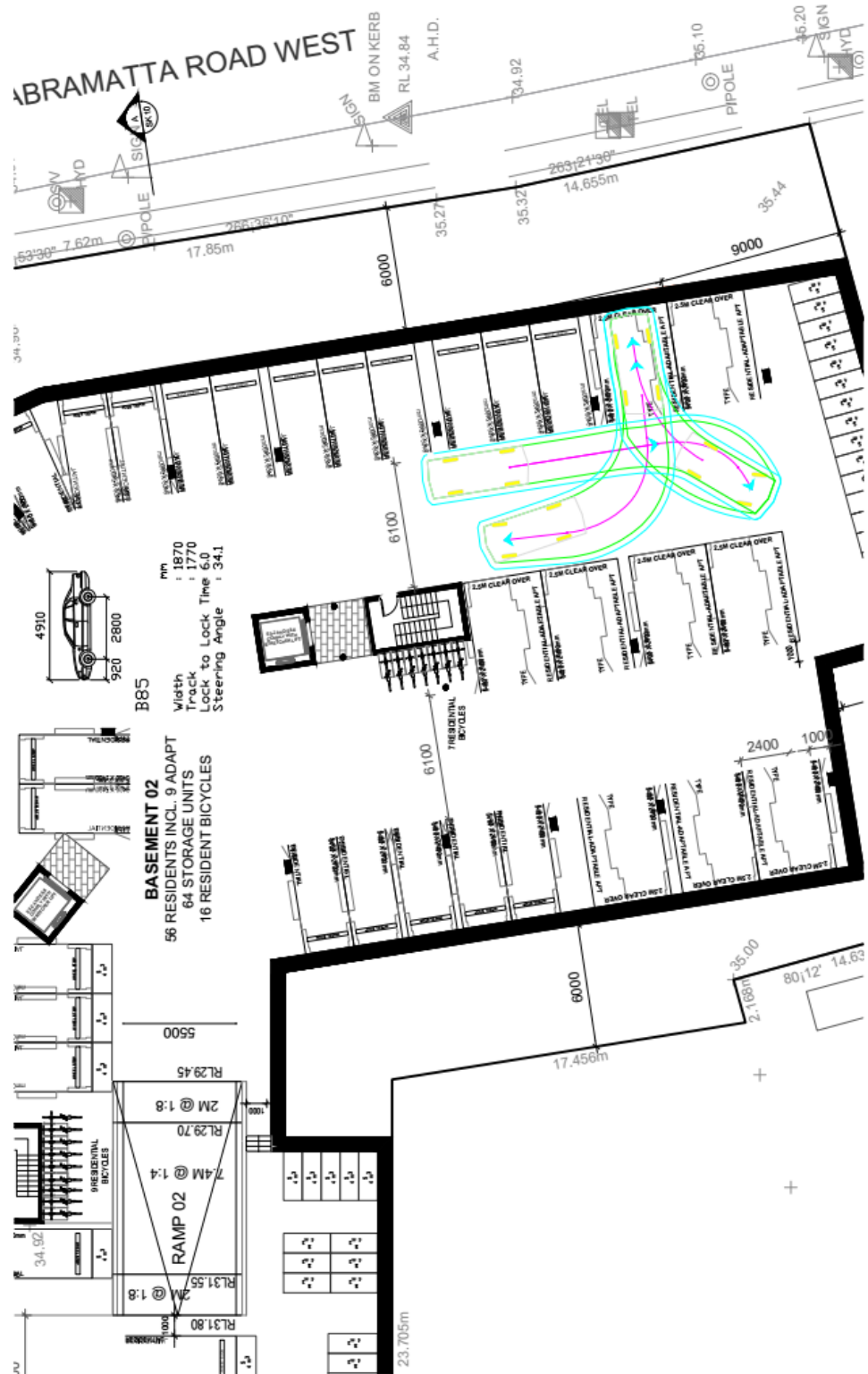


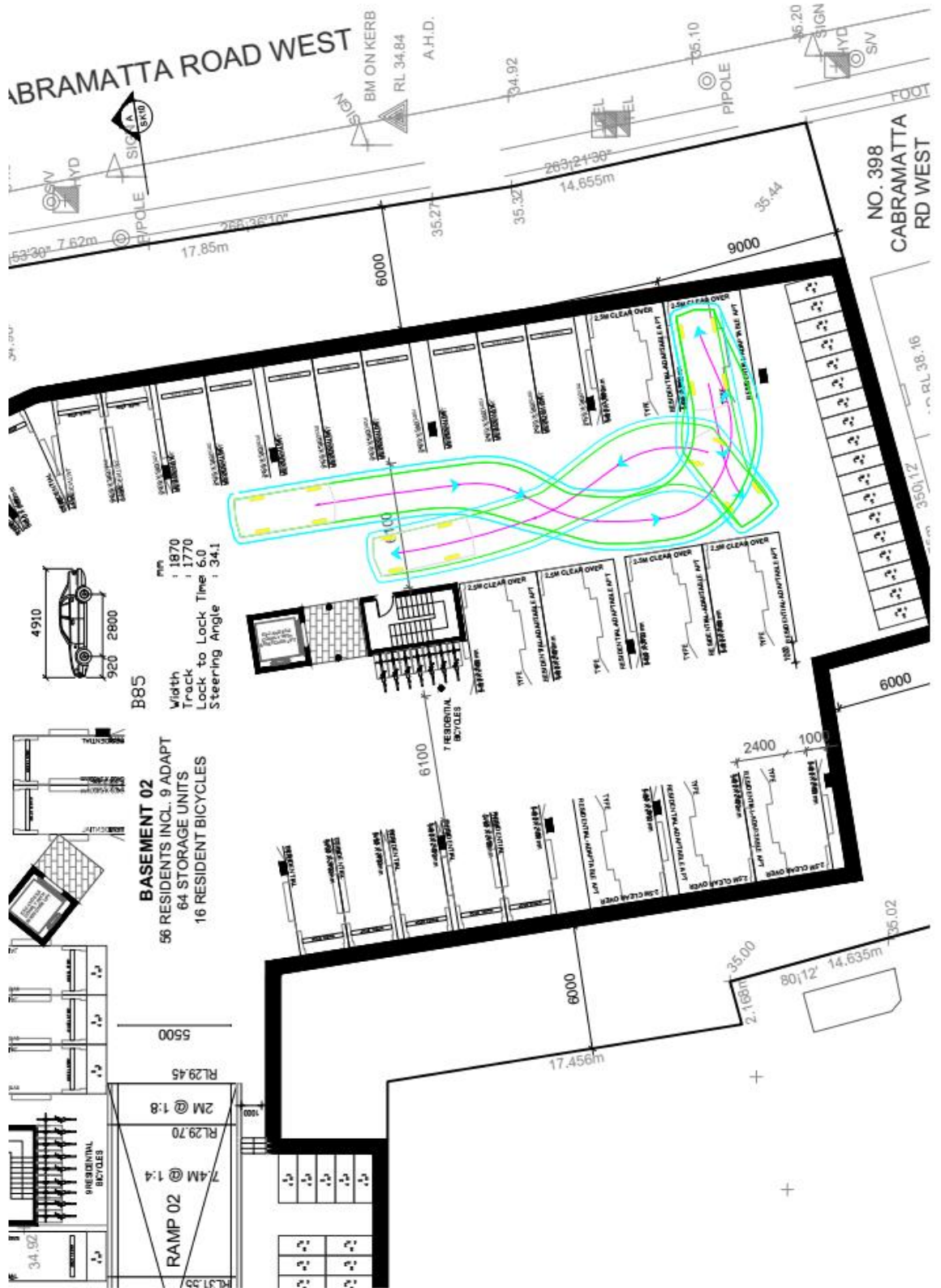


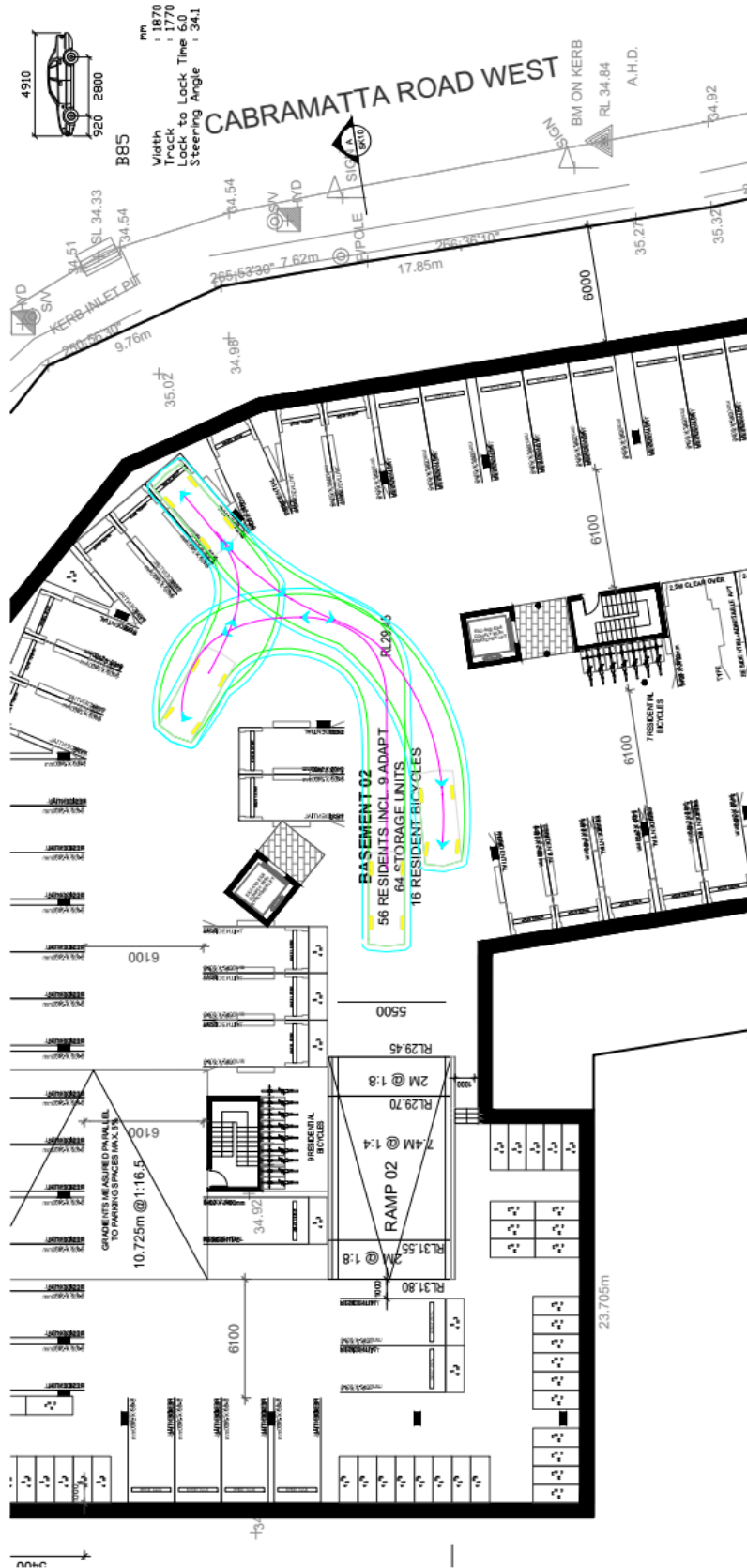


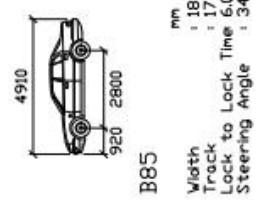


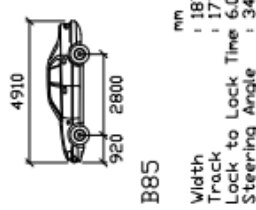


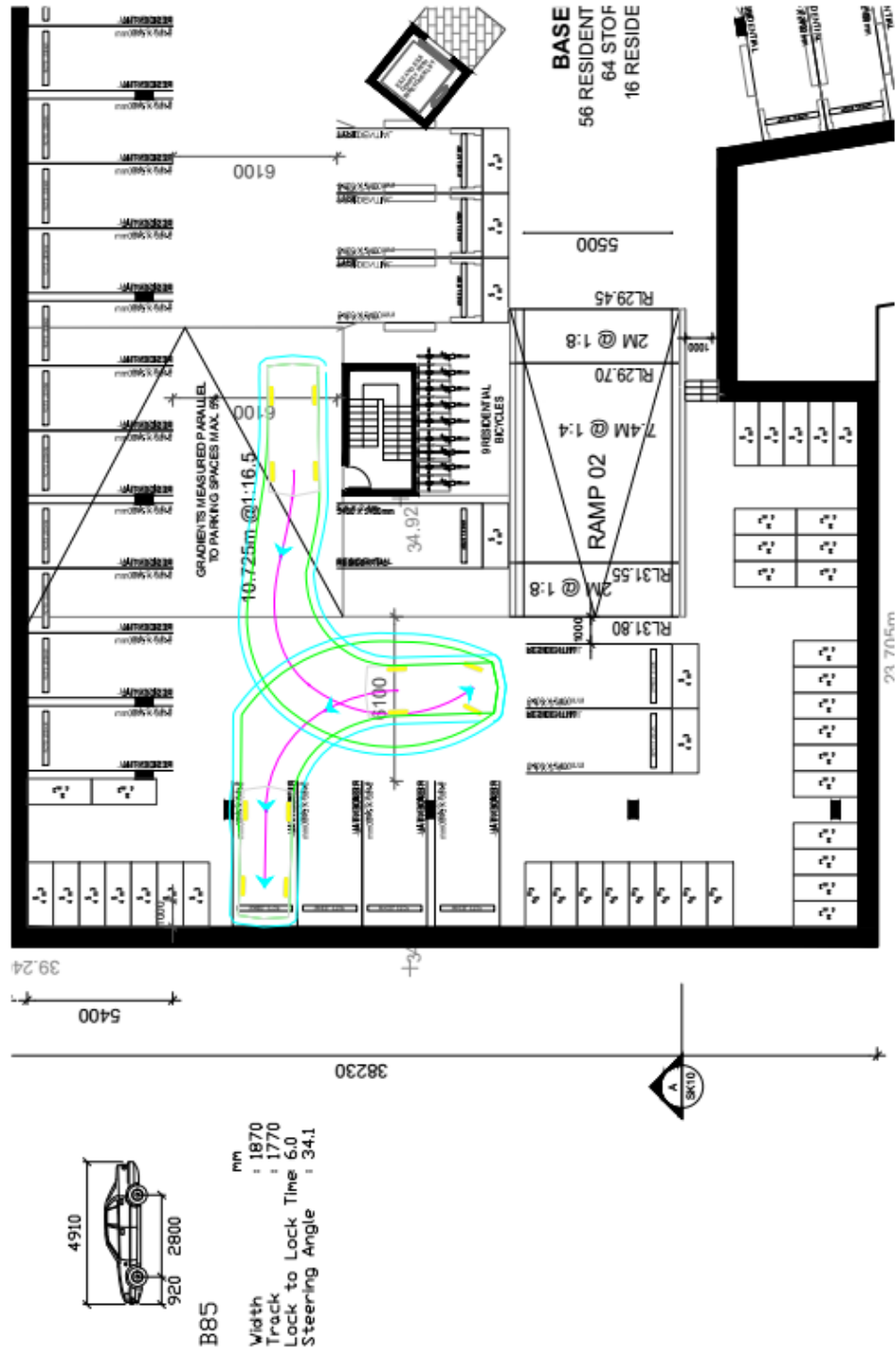


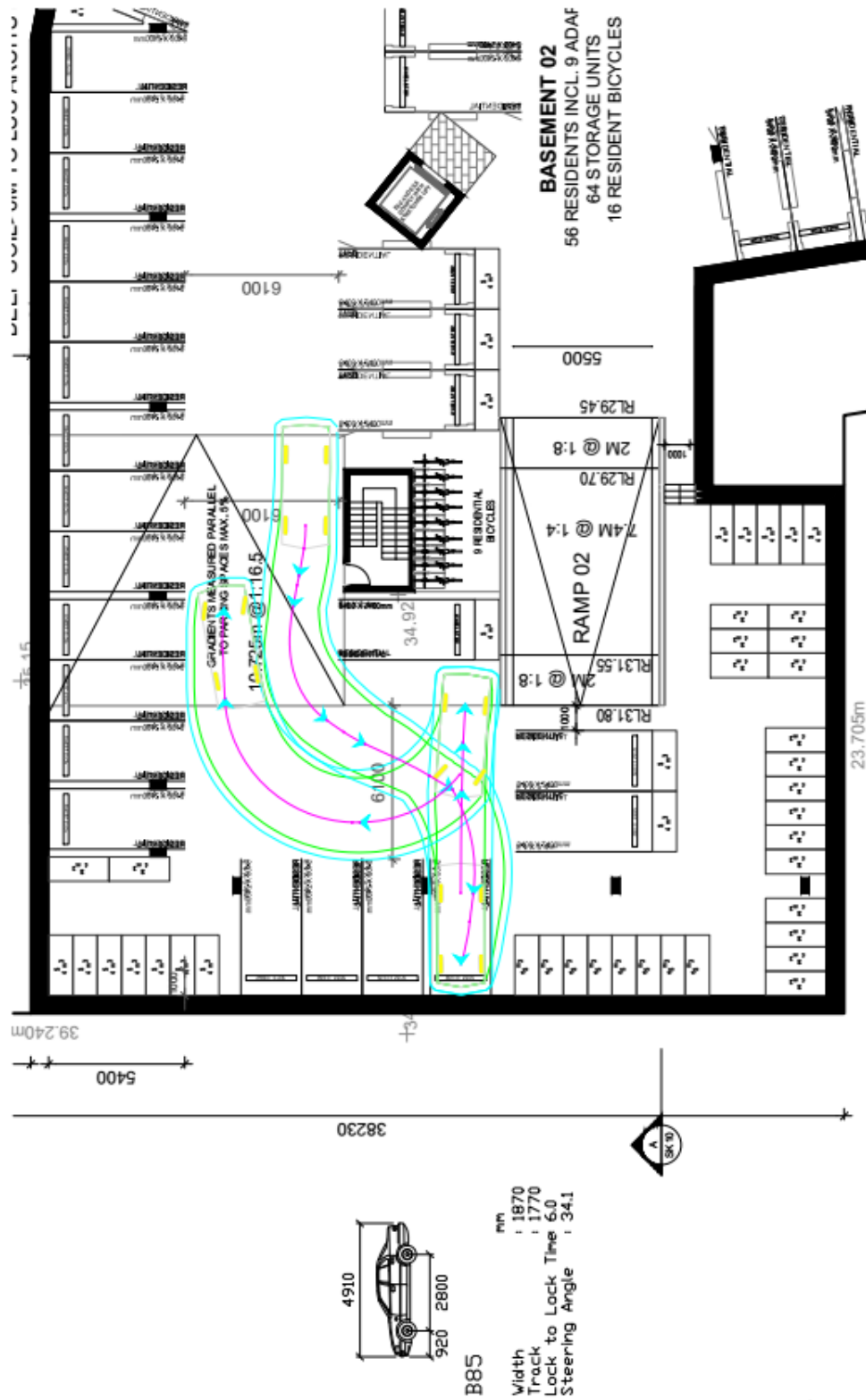


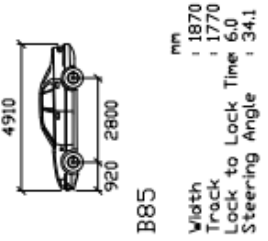


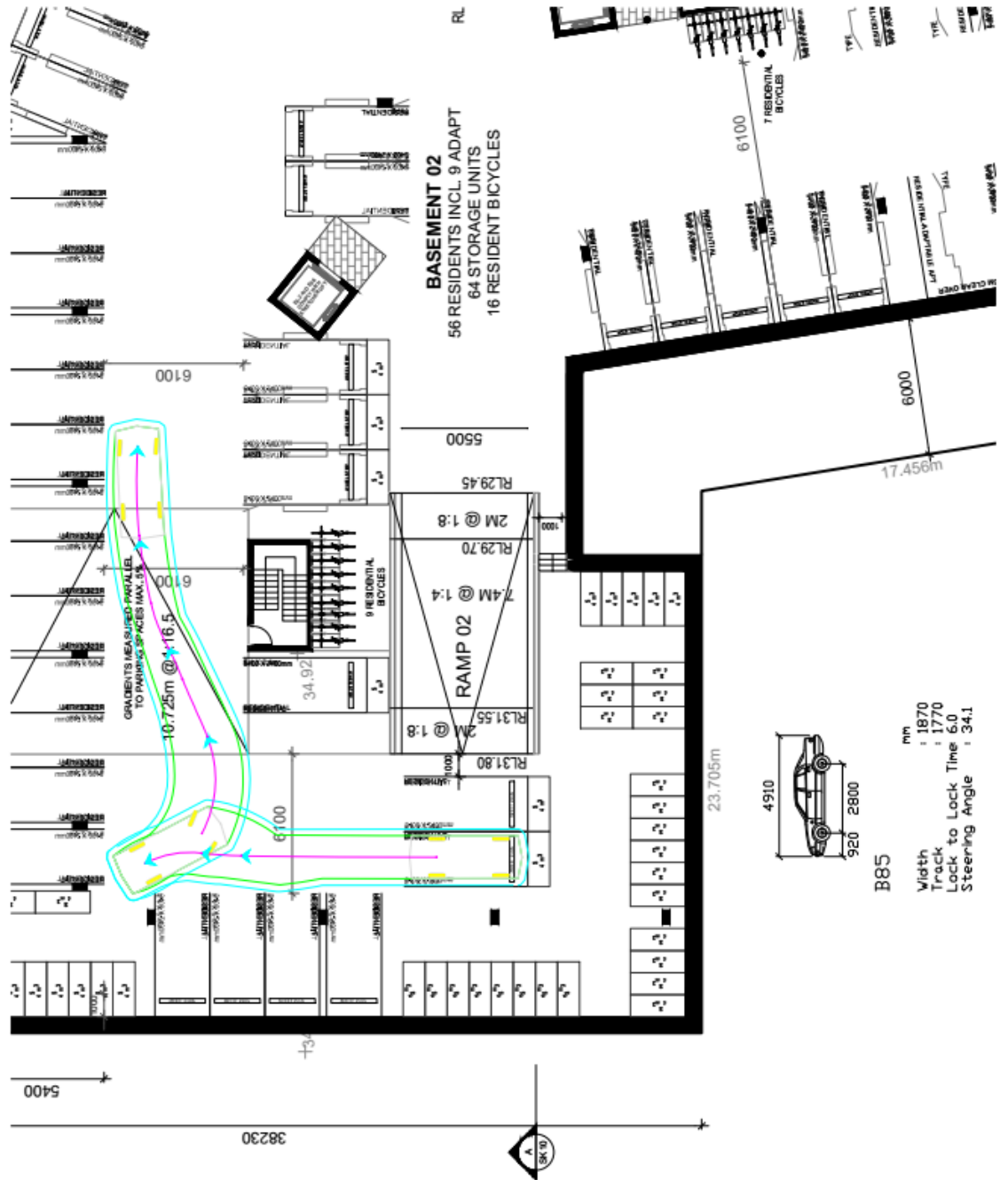


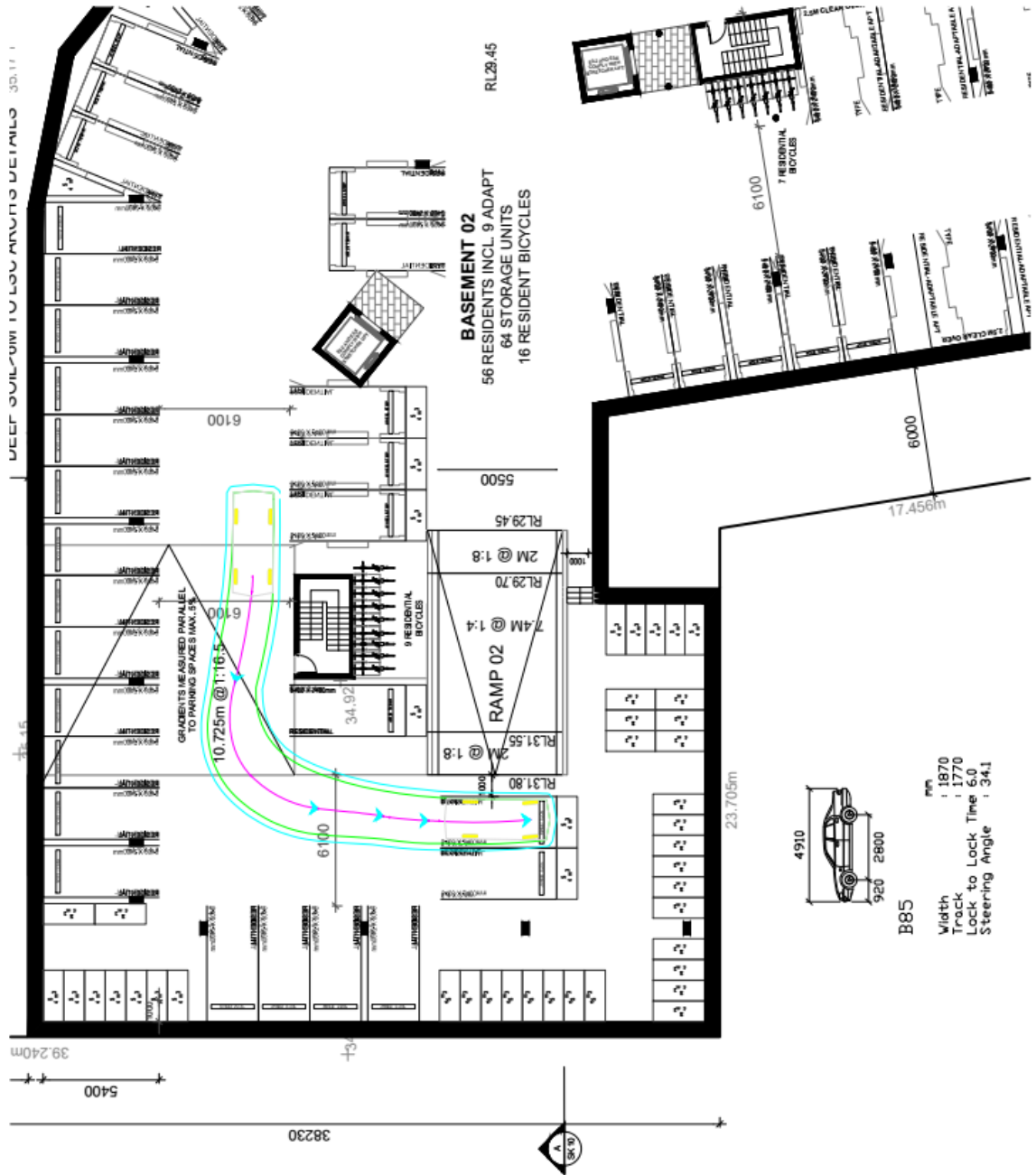


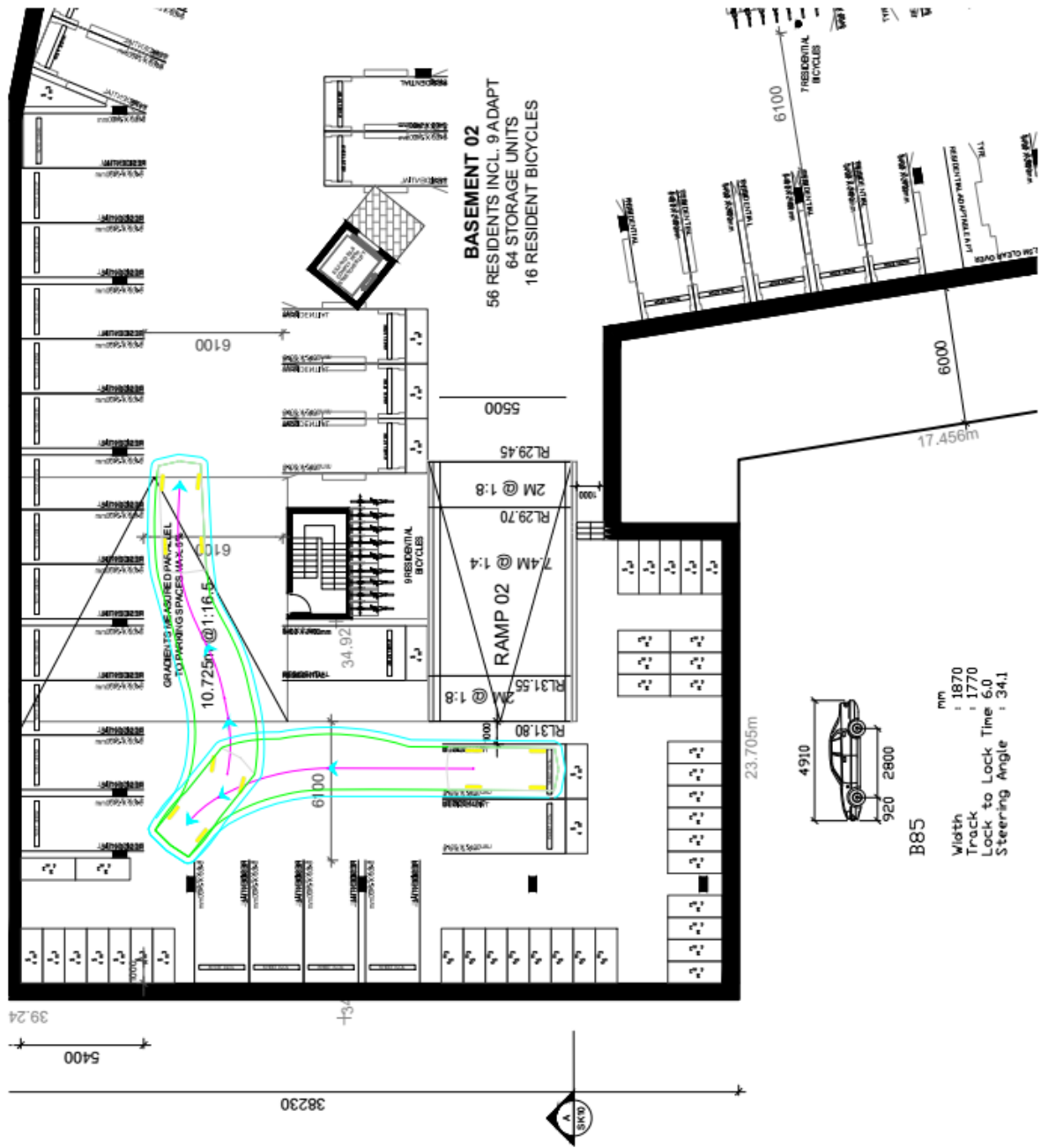


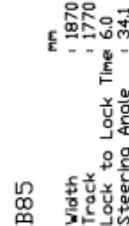




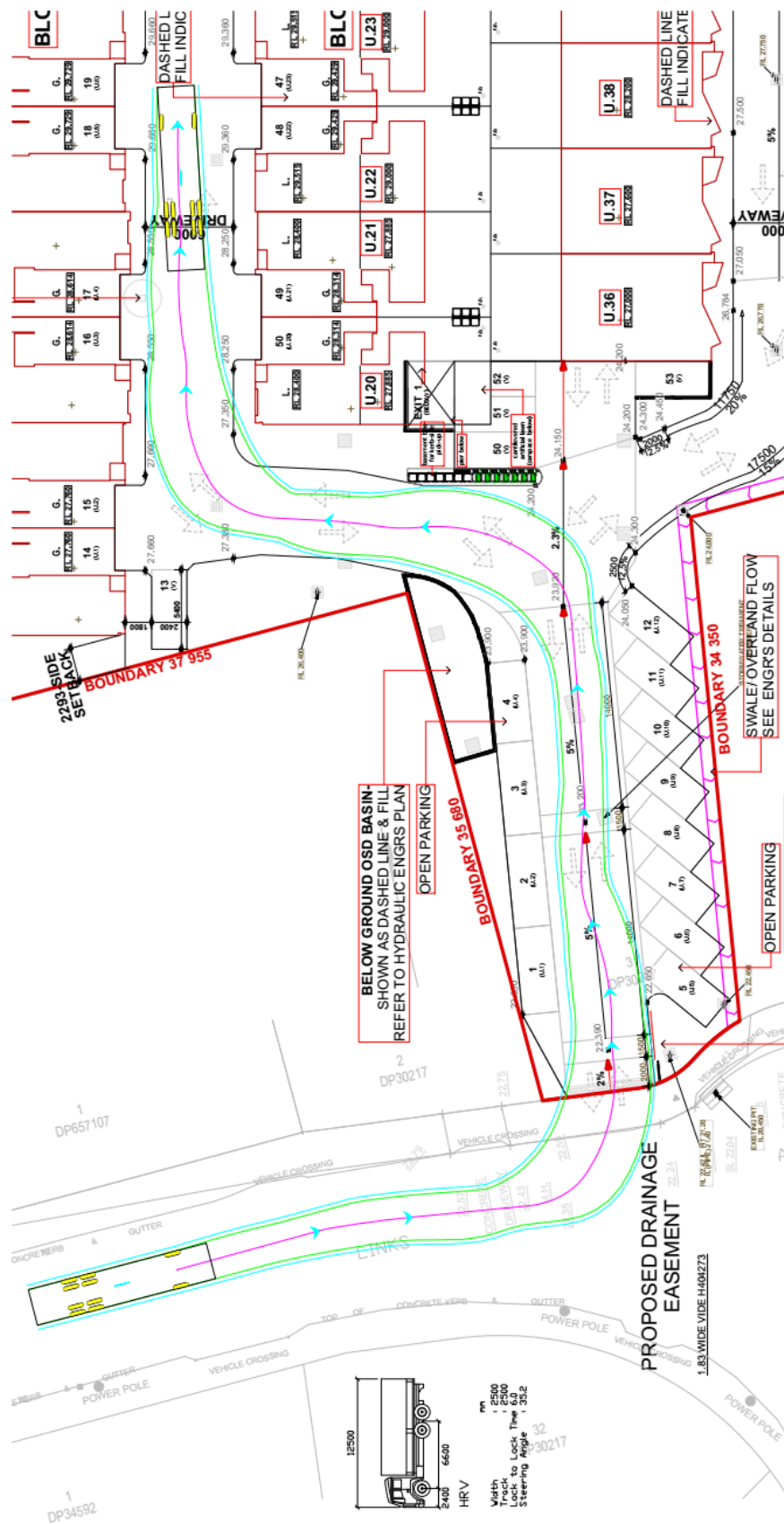


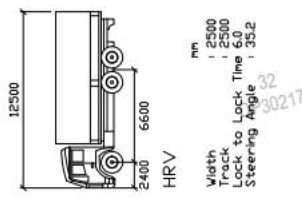




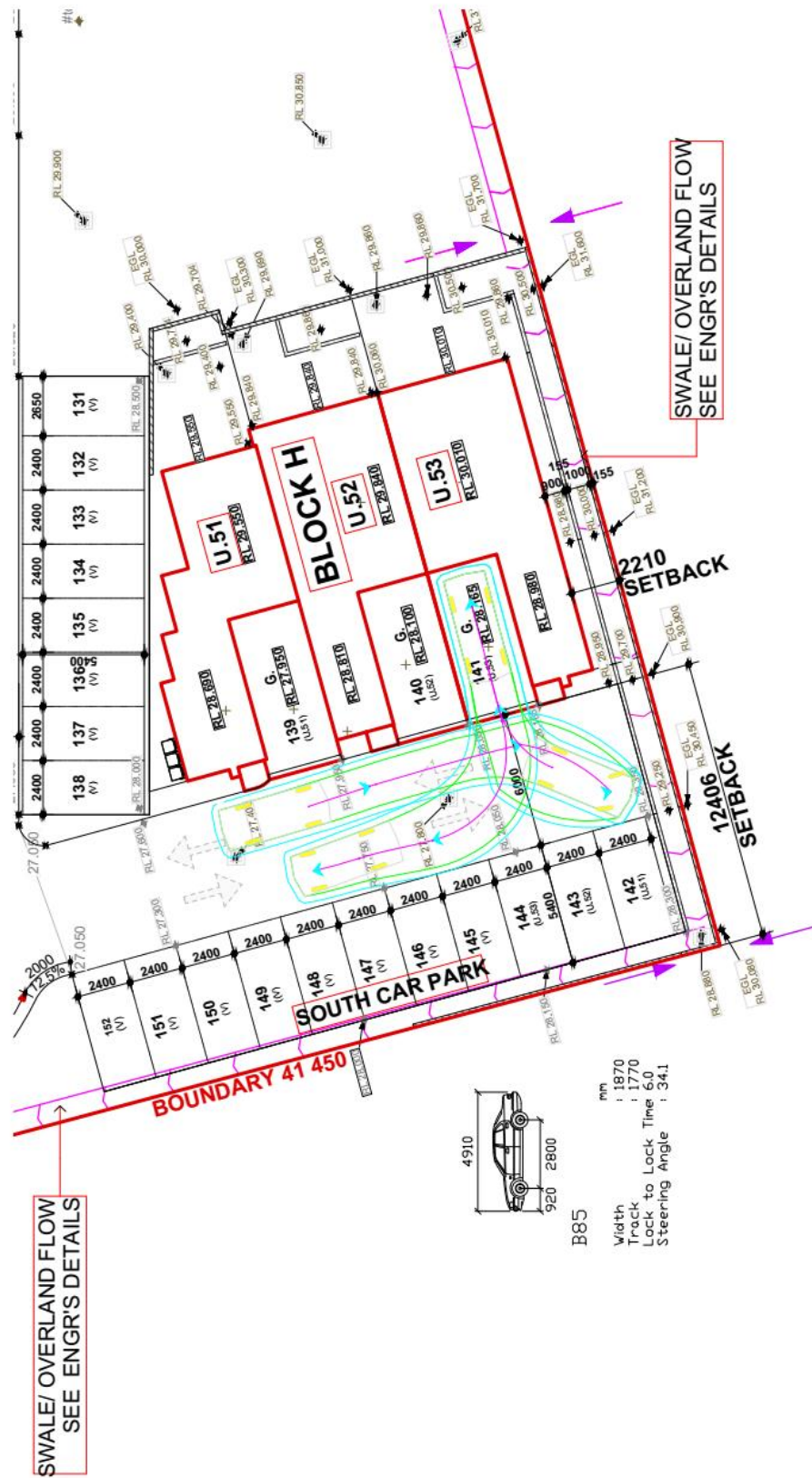


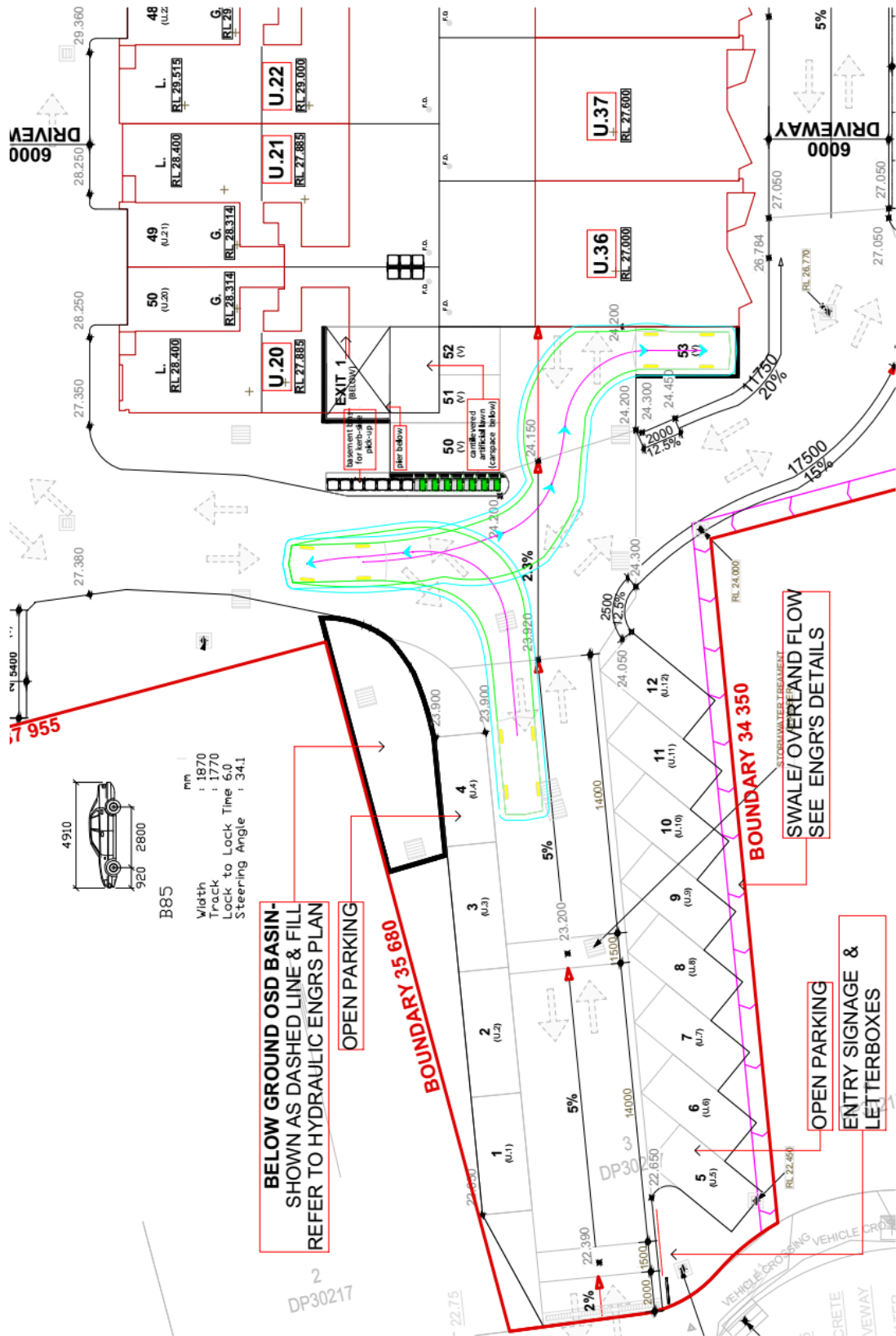
Appendix B – Vehicle Swept Paths (Townhouses)



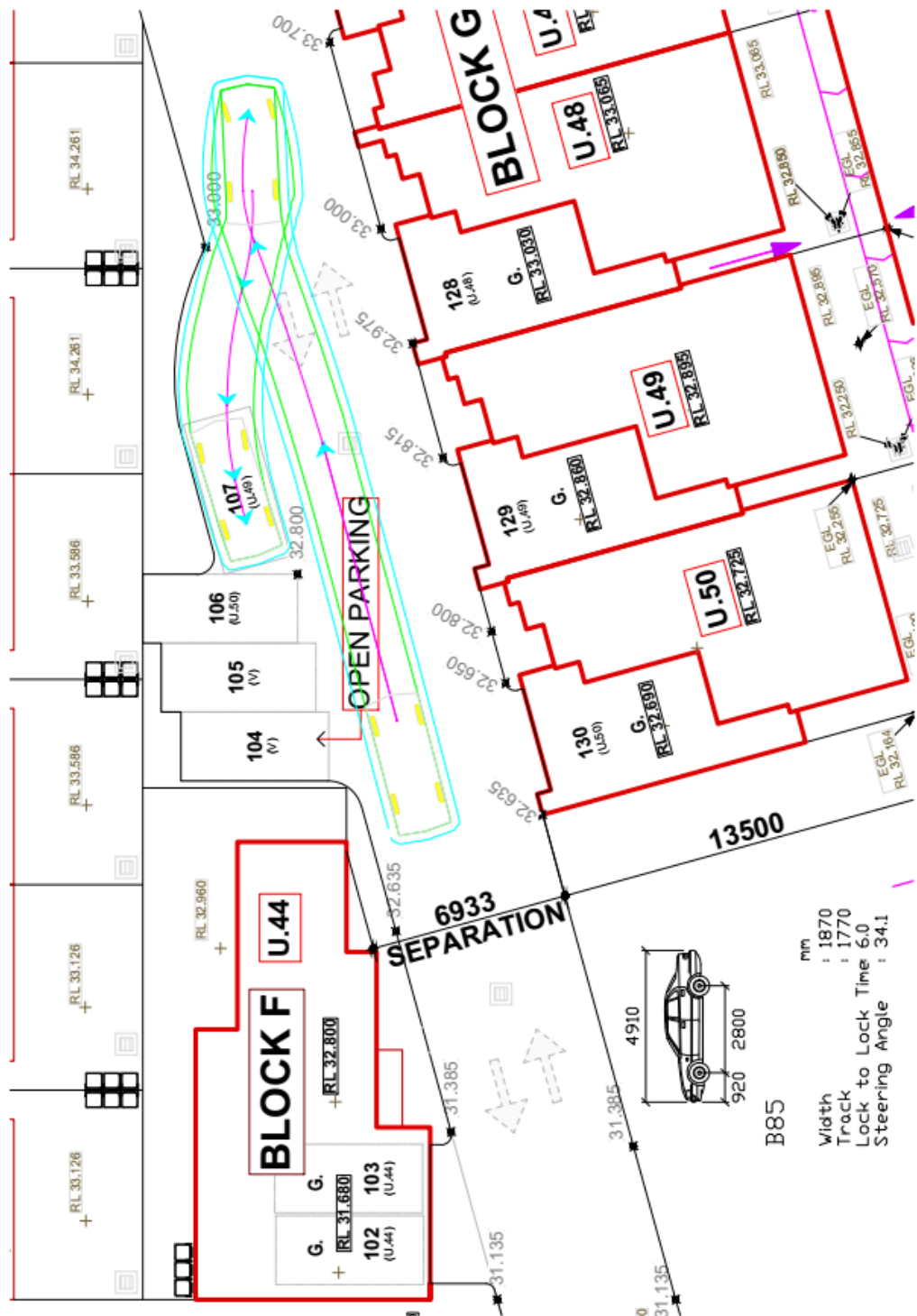


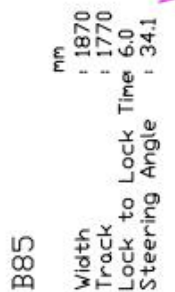


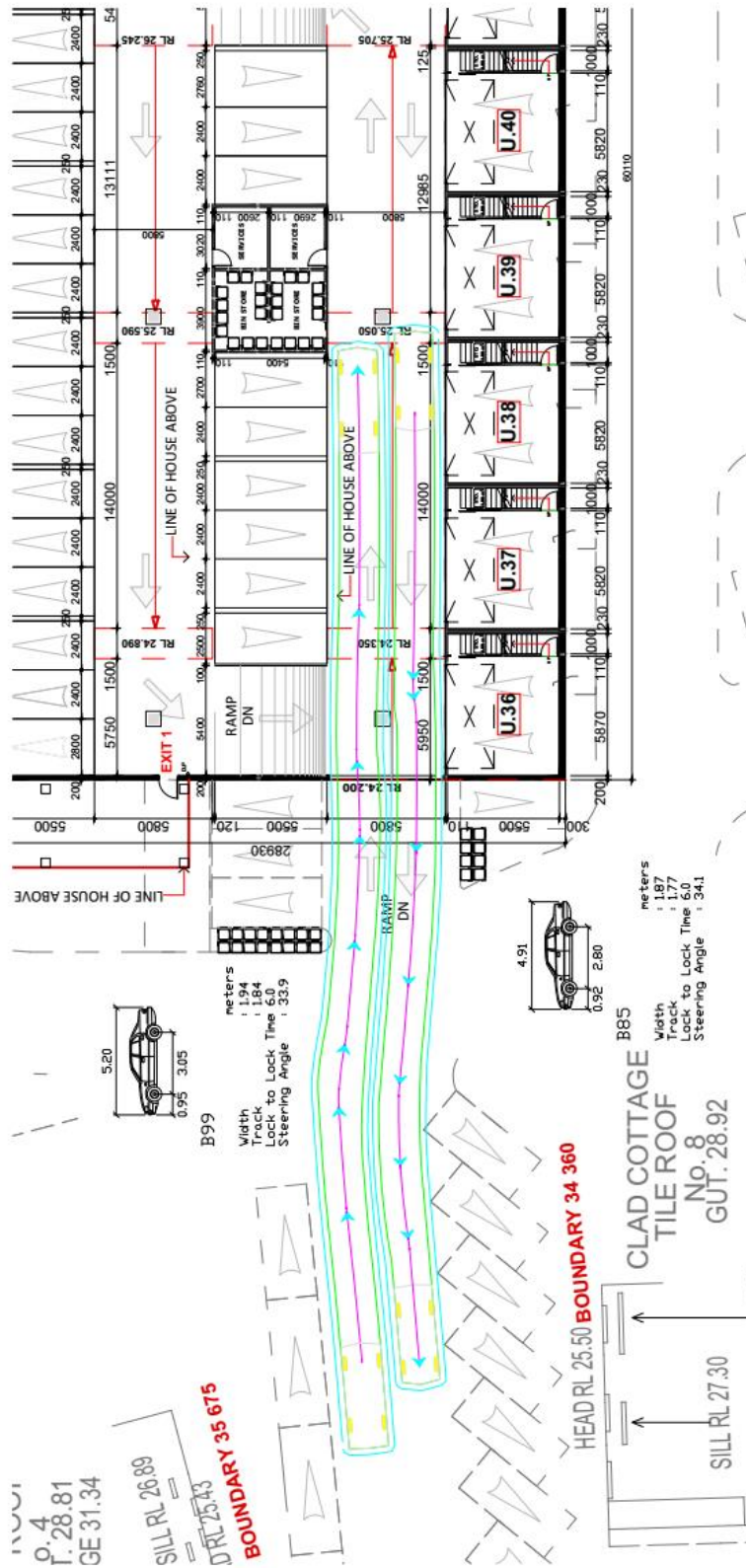


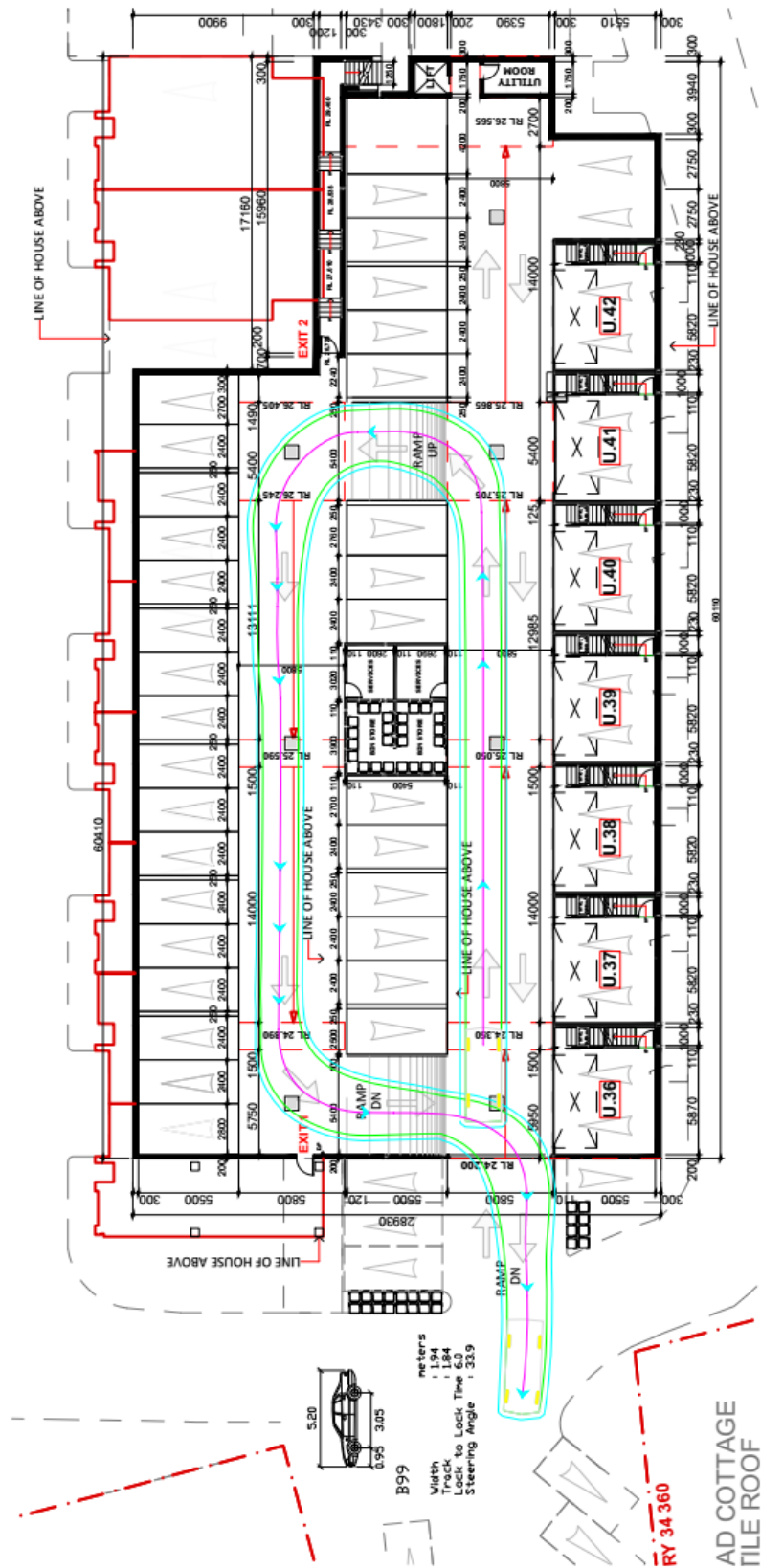




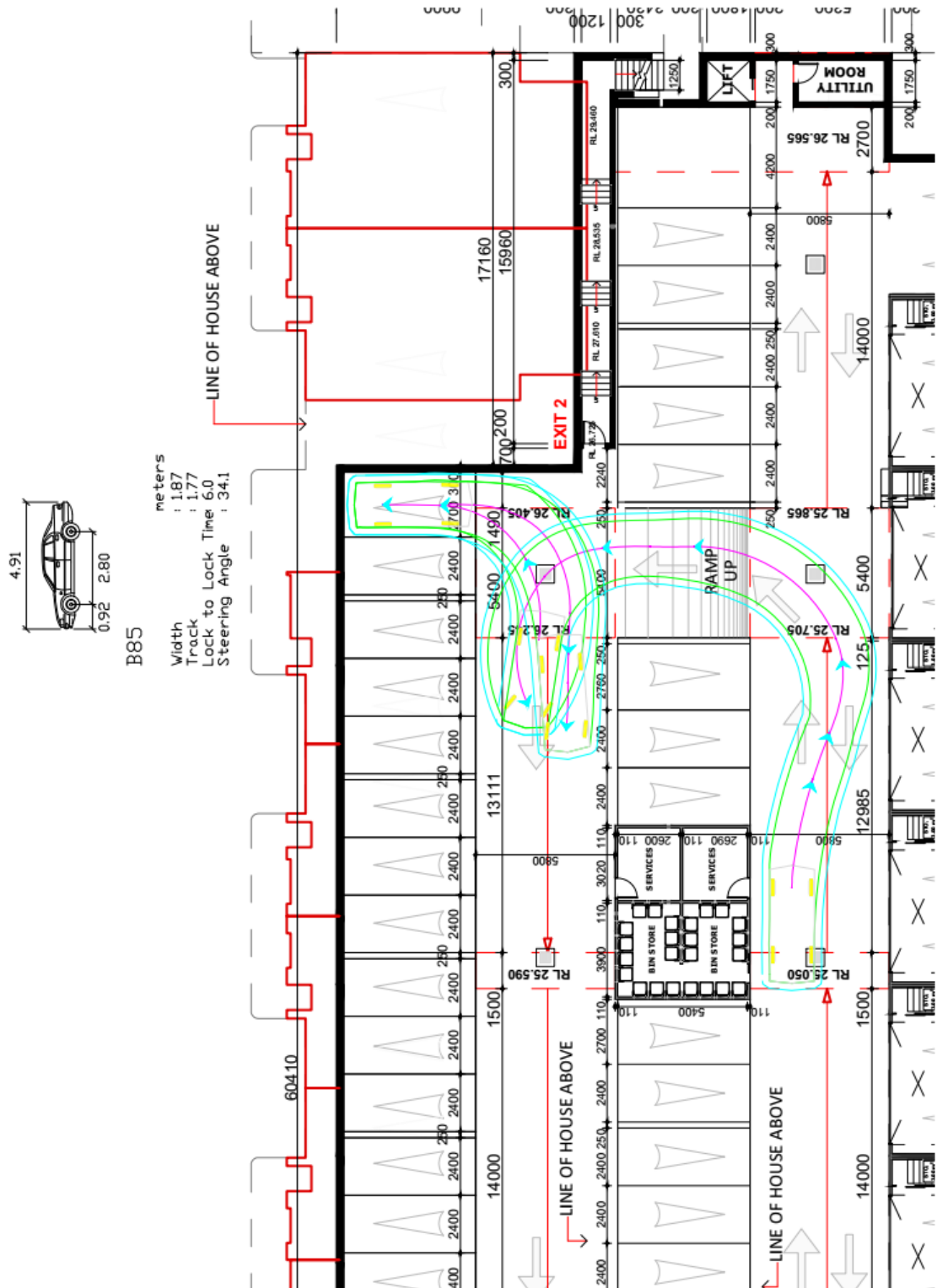




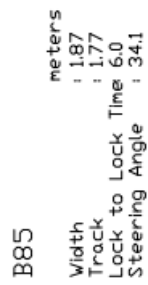


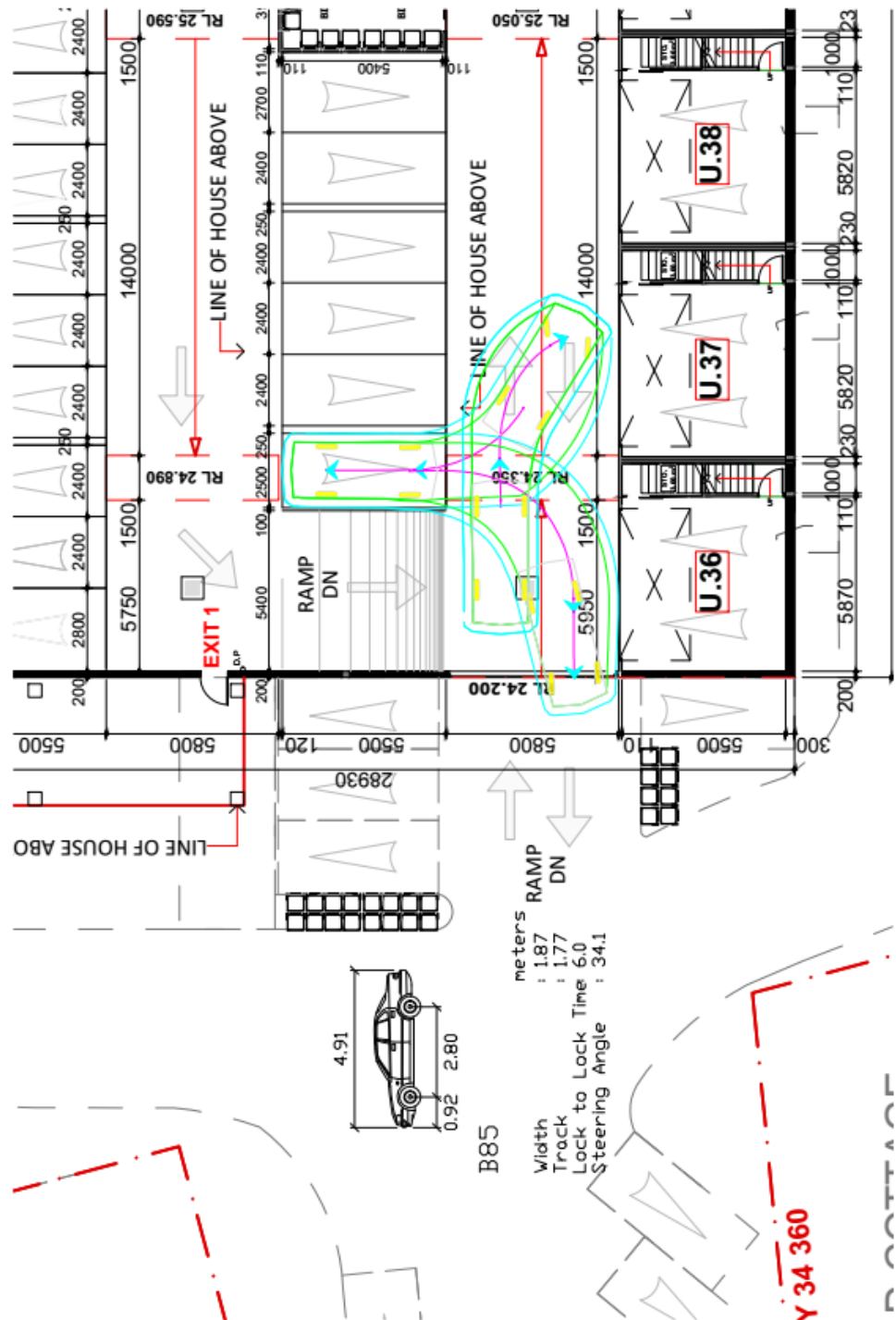


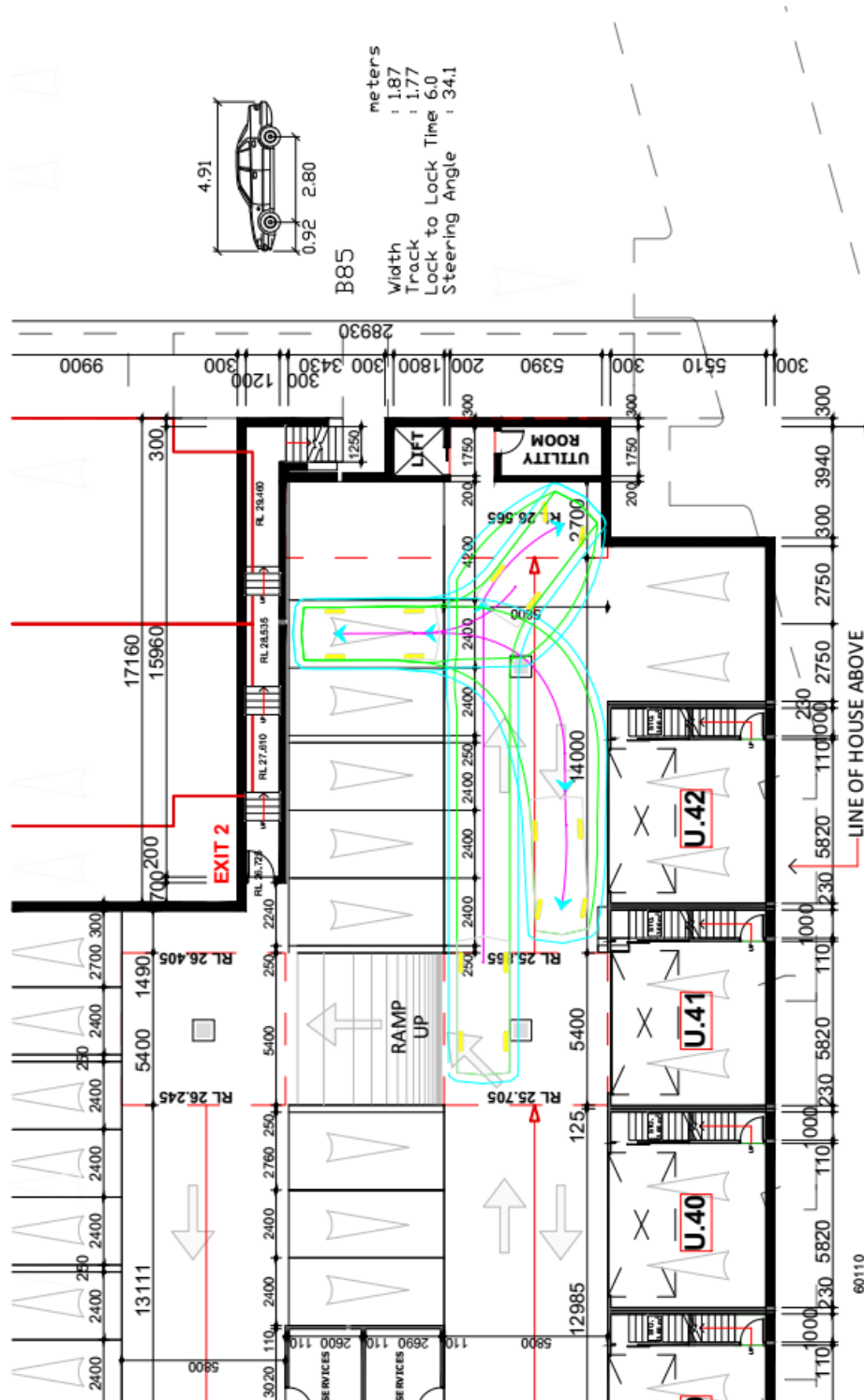












Appendix C – Traffic Volume Counts

Appendix D – SIDRA Intersection Results

MOVEMENT SUMMARY – 7.45am – 8.45am – Base Year 2022 – Orange Grove Road / Links Avenue

Pre-Development

Post-Development

Vehicle Movement Performance												
Move Turn ID	INBOUND VOLUMES [veh/h]	DEMAND FLOWS [Total Flow]	DEMAND FLOWS [Total Flow]	DEMAND FLOWS [Total Flow]	DEMAND FLOWS [Total Flow]	DEMAND FLOWS [Total Flow]	DEMAND FLOWS [Total Flow]	DEMAND FLOWS [Total Flow]	DEMAND FLOWS [Total Flow]	DEMAND FLOWS [Total Flow]	DEMAND FLOWS [Total Flow]	DEMAND FLOWS [Total Flow]
1	2	5	0.0	0.0	0.494	15.3	LOS B	12.2	90.2	0.64	0.56	0.64
2	11	1506	7.0	+0.494	9.0	LOS A	12.2	90.3	0.64	0.56	0.64	0.56
3	R2	1	0.0	0.0	0.002	36.4	LOS C	0.0	0.3	0.71	0.61	0.71
Approach	1512	7.0	1502	6.9	0.494	9.0	LOS A	12.2	90.3	0.64	0.56	0.64
South: Orange Grove Avenue												
4	L2	10	0.0	0.0	0.125	50.0	LOS D	1.7	12.2	0.88	0.72	0.88
5	T1	1	0.0	0.0	+0.125	46.5	LOS D	1.7	12.2	0.88	0.72	0.88
6	R2	22	0.0	0.0	0.125	50.0	LOS D	1.7	12.2	0.88	0.72	0.88
Approach	33	0.0	35	0.0	0.125	49.9	LOS D	1.7	12.2	0.88	0.72	0.88
North: Orange Grove Avenue												
7	L2	10	0.0	0.0	0.880	54.3	LOS D	37.5	278.2	1.00	1.01	1.15
8	T1	1703	7.0	+0.880	48.0	LOS D	37.5	278.4	1.00	1.01	1.15	1.15
9	R2	2	0.0	0.0	+0.023	56.9	LOS E	0.1	0.9	0.97	0.61	0.97
Approach	1715	7.0	1805	7.0	0.880	48.0	LOS D	37.5	278.4	1.00	1.01	1.15
West: Golf Club Private Road												
10	L2	1	0.0	0.0	0.019	47.2	LOS D	0.3	1.8	0.85	0.63	0.85
11	T1	1	0.0	0.0	0.019	44.0	LOS D	0.3	1.8	0.85	0.63	0.85
12	R2	3	0.0	0.0	0.019	47.3	LOS D	0.3	1.8	0.85	0.63	0.85
Approach	5	0.0	5	0.0	0.019	46.6	LOS D	0.3	1.8	0.85	0.63	0.85
All Vehicles	3265	6.9	3437	6.9	0.880	36.0	LOS C	37.5	278.4	0.83	0.80	0.91

Vehicle Movement Performance												
Move Turn ID	INBOUND VOLUMES [veh/h]	DEMAND FLOWS [Total Flow]	DEMAND FLOWS [Total Flow]	DEMAND FLOWS [Total Flow]	DEMAND FLOWS [Total Flow]	DEMAND FLOWS [Total Flow]	DEMAND FLOWS [Total Flow]	DEMAND FLOWS [Total Flow]	DEMAND FLOWS [Total Flow]	DEMAND FLOWS [Total Flow]	DEMAND FLOWS [Total Flow]	DEMAND FLOWS [Total Flow]
1	2	5	0.0	0.0	0.494	15.3	LOS B	12.2	90.2	0.64	0.56	0.64
2	11	1506	7.0	+0.494	9.0	LOS A	12.2	90.3	0.64	0.56	0.64	0.56
3	R2	1	0.0	0.0	0.020	36.9	LOS C	0.5	3.2	0.72	0.68	0.72
Approach	1522	6.9	1502	6.9	0.494	9.2	LOS A	12.2	90.3	0.64	0.56	0.64
East: Links Avenue												
4	L2	25	0.0	0.0	0.318	52.1	LOS D	4.6	32.3	0.92	0.77	0.92
5	T1	1	0.0	0.0	+0.318	47.5	LOS D	4.6	32.3	0.92	0.77	0.92
6	R2	58	0.0	0.0	0.318	52.0	LOS D	4.6	32.3	0.92	0.77	0.92
Approach	84	0.0	88	0.0	0.318	52.0	LOS D	4.6	32.3	0.92	0.77	0.92
North: Orange Grove Avenue												
7	L2	51	0.0	0.0	0.901	58.4	LOS E	40.3	297.4	1.00	1.04	1.19
8	T1	1703	7.0	+0.901	52.0	LOS D	40.3	298.9	1.00	1.04	1.19	1.19
9	R2	2	0.0	0.0	+0.023	56.9	LOS E	0.1	0.9	0.97	0.61	0.97
Approach	1756	6.8	1840	6.8	0.901	52.2	LOS D	40.3	298.9	1.00	1.04	1.19
West: Golf Club Private Road												
10	L2	1	0.0	0.0	0.019	47.3	LOS D	0.3	1.8	0.85	0.63	0.85
11	T1	1	0.0	0.0	0.019	44.1	LOS D	0.3	1.8	0.85	0.63	0.85
12	R2	3	0.0	0.0	0.019	47.3	LOS D	0.3	1.8	0.85	0.63	0.85
Approach	5	0.0	5	0.0	0.019	46.6	LOS D	0.3	1.8	0.85	0.63	0.85
All Vehicles	3367	6.7	3544	6.7	0.901	32.8	LOS C	40.3	298.9	0.83	0.82	0.93

MOVEMENT SUMMARY – 3.45pm – 4.45pm – Base Year 2022 – Orange Grove Road / Links Avenue

Pre-Development

Post-Development

Vehicle Movement Performance													
Mov. Turn ID	INPUT VOLUMES [Total RTV] with %		DEMAND FLOWS [Total RTV] with %		Des. Satn	Ave. Level of Delay Service	50% BACK OF QUEUE [Veh. Del] with m	Prog. Effective Que	Stop Rate	Ave. No. Cycles	Ave. No. Stop Cycles	Ave. No. Speed km/h	
South, Orange Grove Avenue													
1	L2	3	0.0	3	0.0	0.557	15.1 LOS-B	18.0	133.6	0.60	0.54	0.60	42.9
2	T1	1911	7.0	2012	7.0	+0.557	8.7 LOS-A	18.0	133.6	0.60	0.54	0.60	59.1
3	R2	18	0.0	19	0.0	0.030	41.0 LOS-C	0.9	6.3	0.69	0.69	0.69	35.7
Approach 1922 6.9 2034 6.9													
						0.557	9.0 LOS-A	18.0	133.6	0.60	0.54	0.60	58.7
East Links Avenue													
4	L2	19	0.0	20	0.0	0.203	65.9 LOS-E	4.5	31.5	0.93	0.76	0.93	27.7
5	T1	1	0.0	1	0.0	+0.203	61.4 LOS-E	4.5	31.5	0.93	0.76	0.93	11.0
6	R2	45	0.0	47	0.0	0.203	65.9 LOS-E	4.5	31.5	0.93	0.76	0.93	16.3
Approach 66 0.0 68 0.0													
						0.203	65.9 LOS-E	4.5	31.5	0.93	0.76	0.93	21.3
North, Orange Grove Avenue													
7	L2	58	0.0	72	0.0	0.919	69.3 LOS-E	53.8	399.5	1.00	1.03	1.17	18.8
8	T1	1819	7.0	1915	7.0	+0.919	62.4 LOS-E	53.8	399.1	1.00	1.03	1.17	29.9
9	R2	2	0.0	2	0.0	+0.028	83.6 LOS-F	0.2	1.1	0.96	0.61	0.96	10.3
Approach 1069 6.7 1966 6.7													
						0.919	63.2 LOS-E	53.8	399.1	1.00	1.03	1.17	29.6
West Golf Club Private Road													
10	L2	2	0.0	2	0.0	0.016	60.7 LOS-E	0.3	1.8	0.87	0.62	0.87	13.3
11	T1	1	0.0	1	0.0	0.016	57.5 LOS-E	0.3	1.8	0.87	0.62	0.87	14.4
12	R2	1	0.0	1	0.0	0.016	60.7 LOS-E	0.3	1.8	0.87	0.62	0.87	26.0
Approach 4 0.0 4 0.0													
						0.016	59.9 LOS-E	0.3	1.8	0.87	0.62	0.87	17.4
All Vehicles													
						0.919	36.3 LOS-C	53.8	399.1	0.80	0.78	0.88	39.2

Vehicle Movement Performance													
Mov. Turn ID	INPUT VOLUMES [Total RTV] with %		DEMAND FLOWS [Total RTV] with %		Des. Satn	Ave. Level of Delay Service	50% BACK OF QUEUE [Veh. Del] with m	Prog. Effective Que	Stop Rate	Ave. No. Cycles	Ave. No. Stop Cycles	Ave. No. Speed km/h	
South, Orange Grove Avenue													
1	L2	3	0.0	3	0.0	0.557	15.1 LOS-B	18.0	133.6	0.60	0.54	0.60	42.9
2	T1	1911	7.0	2012	7.0	+0.557	8.7 LOS-A	18.0	133.6	0.60	0.54	0.60	59.1
3	R2	18	0.0	19	0.0	0.030	40.7 LOS-C	0.9	6.3	0.68	0.67	0.68	35.7
Approach 1922 6.9 2023 6.9													
						0.557	8.8 LOS-A	18.0	133.6	0.60	0.54	0.60	58.9
East Links Avenue													
4	L2	5	0.0	5	0.0	0.074	63.3 LOS-E	1.1	7.9	0.89	0.69	0.89	28.3
5	T1	1	0.0	1	0.0	+0.074	58.7 LOS-E	1.1	7.9	0.89	0.69	0.89	12.2
6	R2	11	0.0	12	0.0	0.074	63.3 LOS-E	1.1	7.9	0.89	0.69	0.89	18.8
Approach 17 0.0 18 0.0													
						0.074	63.0 LOS-E	1.1	7.9	0.89	0.69	0.89	21.7
North, Orange Grove Avenue													
7	L2	30	0.0	32	0.0	0.900	64.5 LOS-E	50.4	372.8	1.00	1.00	1.13	19.9
8	T1	1819	7.0	1915	7.0	+0.900	58.1 LOS-E	50.4	373.9	1.00	1.01	1.13	31.3
9	R2	2	0.0	2	0.0	+0.028	83.6 LOS-F	0.2	1.1	0.96	0.61	0.96	10.3
Approach 1061 6.9 1940 6.9													
						0.900	58.2 LOS-E	50.4	373.9	1.00	1.01	1.13	31.2
West Golf Club Private Road													
10	L2	2	0.0	2	0.0	0.016	60.7 LOS-E	0.3	1.8	0.87	0.62	0.87	13.3
11	T1	1	0.0	1	0.0	0.016	57.5 LOS-E	0.3	1.8	0.87	0.62	0.87	14.4
12	R2	1	0.0	1	0.0	0.016	60.7 LOS-E	0.3	1.8	0.87	0.62	0.87	26.0
Approach 4 0.0 4 0.0													
						0.016	59.9 LOS-E	0.3	1.8	0.87	0.62	0.87	17.4
All Vehicles													
						0.900	33.2 LOS-C	50.4	373.9	0.79	0.77	0.86	40.9

MOVEMENT SUMMARY – 7.45am – 8.45am – Future Year 2032 – Orange Grove Road / Links Avenue

Pre-Development

Post-Development

Vehicle Movement Performance																													
Mov. ID	Turn	ABUT	VOLUMES	DEMAND	DEP	Avg. Level of	95% BACK OF	PROP. EFFECTIVE	Avg. Speed	Avg. Delay	Queue	Rate	Cycle	Rate	Cycle														
			[Total HV]	[Total HV]	vc	sec	[Veh. Def]		km/h	sec	m																		
South: Orange Grove Avenue																													
1	L2	5	0.0	6	0.0	0.533	15.2 LOS B	0.59	0.53	0.59	42.8																		
2	T1	1506	7.0	1902	7.0	*0.533	8.9 LOS A	0.59	0.53	0.59	58.9																		
3	R2	11	0.0	14	0.0	0.002	42.4 LOS C	0.1	0.4	0.70	0.62	70	25.0																
Approach			1512	7.0	1910	7.0	0.533	8.9 LOS A	0.59	0.53	58.8																		
East: Links Avenue																													
4	L2	10	0.0	13	0.0	0.165	63.6 LOS E	2.7	18.6	0.90	0.73	0.90	20.2																
5	T1	1	0.0	1	0.0	*0.165	59.0 LOS E	2.7	18.6	0.90	0.73	0.90	12.1																
6	R2	22	0.0	28	0.0	0.165	63.5 LOS E	2.7	18.6	0.90	0.73	0.90	18.7																
Approach			33	0.0	42	0.0	0.165	63.4 LOS E	2.7	18.6	0.90	0.73	0.90	21.9															
North: Orange Grove Avenue																													
7	L2	10	0.0	13	0.0	0.960	86.8 LOS F	66.9	495.8	1.00	1.14	1.29	16.0																
8	T1	1703	7.0	2151	7.0	*0.960	80.4 LOS F	66.9	496.4	1.00	1.14	1.29	25.9																
9	R2	2	0.0	3	0.0	*0.034	83.7 LOS F	0.2	1.3	0.98	0.62	0.98	10.3																
Approach			1715	7.0	2166	7.0	0.960	80.4 LOS F	66.9	496.4	1.00	1.14	1.29	25.8															
West: Golf Club Private Road																													
10	L2	1	0.0	1	0.0	0.025	60.1 LOS E	0.4	2.7	0.87	0.64	0.87	13.4																
11	T1	1	0.0	1	0.0	0.025	56.5 LOS E	0.4	2.7	0.87	0.64	0.87	14.5																
12	R2	3	0.0	4	0.0	0.025	60.2 LOS E	0.4	2.7	0.87	0.64	0.87	26.1																
Approach			5	0.0	6	0.0	0.025	59.5 LOS E	0.4	2.7	0.87	0.64	0.87	22.0															
All Vehicles			3065	6.9	4124	6.9	0.968	47.1 LOS D	0.81	0.85	49.4	0.85	34.9																

Vehicle Movement Performance																													
Mov. ID	Turn	ABUT	VOLUMES	DEMAND	DEP	Avg. Level of	95% BACK OF	PROP. EFFECTIVE	Avg. Speed	Avg. Delay	Queue	Rate	Cycle	Rate	Cycle														
			[Total HV]	[Total HV]	vc	sec	[Veh. Def]		km/h	sec	m																		
South: Orange Grove Avenue																													
1	L2	5	0.0	6	0.0	0.533	15.2 LOS B	0.59	0.53	0.59	42.8																		
2	T1	1506	7.0	1902	7.0	*0.533	8.9 LOS A	0.59	0.53	0.59	58.9																		
3	R2	11	0.0	14	0.0	0.024	43.8 LOS D	0.71	4.6	0.72	0.66	0.72	34.6																
Approach			1522	6.9	1923	6.9	0.533	9.1 LOS A	0.59	0.53	58.9																		
East: Links Avenue																													
4	L2	25	0.0	32	0.0	0.418	56.6 LOS E	7.1	49.7	0.96	0.79	0.96	27.6																
5	T1	1	0.0	1	0.0	*0.418	52.0 LOS E	7.1	49.7	0.96	0.79	0.96	11.7																
6	R2	58	0.0	73	0.0	0.418	56.6 LOS E	7.1	49.7	0.96	0.79	0.96	18.1																
Approach			84	0.0	106	0.0	0.418	56.5 LOS E	7.1	49.7	0.96	0.79	0.96	21.3															
North: Orange Grove Avenue																													
7	L2	51	0.0	64	0.0	0.975	89.7 LOS F	69.9	516.0	1.00	1.14	1.30	15.5																
8	T1	1703	7.0	2151	7.0	*0.975	83.4 LOS F	69.9	518.8	1.00	1.15	1.30	25.3																
9	R2	2	0.0	3	0.0	*0.034	83.7 LOS F	0.2	1.3	0.98	0.62	0.98	10.3																
Approach			1756	6.8	2218	6.8	0.975	83.5 LOS F	69.9	518.8	1.00	1.15	1.30	25.0															
West: Golf Club Private Road																													
10	L2	1	0.0	1	0.0	0.026	61.1 LOS E	0.4	2.7	0.87	0.64	0.87	13.2																
11	T1	1	0.0	1	0.0	0.026	57.9 LOS E	0.4	2.7	0.87	0.64	0.87	14.3																
12	R2	3	0.0	4	0.0	0.026	61.1 LOS E	0.4	2.7	0.87	0.64	0.87	25.9																
Approach			5	0.0	6	0.0	0.026	60.5 LOS E	0.4	2.7	0.87	0.64	0.87	21.8															
All Vehicles			3367	6.7	4253	6.7	0.975	49.4 LOS D	0.81	0.86	518.8	0.81	0.86	33.8															

MOVEMENT SUMMARY – 3.45pm – 4.45pm – Future Year 2032 – Orange Grove Road / Links Avenue

Pre-Development

Post-Development